
TOWN OF WILLISTON

TOWN-WIDE WATERSHED IMPROVEMENT PLAN - PHASE 1

ALLEN BROOK

MUDDY BROOK

SUCKER BROOK

WINOOSKI RIVER

FINAL REPORT

Stone Project ID 12-055

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1. INTRODUCTION

The Town of Williston is a vibrant community located in Chittenden County, Vermont. Over the past half century, Williston has slowly transformed from a small village surrounded by working farms to a multi-faceted community with dynamic residential, commercial, and industrial activity as well as valuable open space, rural areas, and plentiful natural resources. The Town has taken a proactive approach to community planning that includes both utilization and protection of land and water resources. As noted in the 2011 Town Plan, “the town has a strong history of using the planning process to moderate the impacts of population growth and land use change on the environment, public facilities and services, and the character of the community.”

As development has increased within the Town of Williston, so has the complexity of managing the Town’s water resources. The Town of Williston faces a range of regulatory and non-regulatory issues related to surface water protection and stormwater management. Two

of the Town’s major streams, Allen Brook and Muddy Brook, are considered impaired by contaminants primarily from storm runoff (see sidebar). Lake Champlain, which is the ultimate receiving water stream and rivers leaving for the Town, is also considered to be impaired. Areas of Williston are experiencing increased stormwater runoff volumes as a result of development; localized stream channel and land erosion concerns are problematic in their own right, but also contribute phosphorus to Lake Champlain.

Stormwater runoff is commonly transported through Municipal Separate Storm Sewer Systems (MS4s), from which it is often discharged untreated into local receiving waters. The most effective strategy for reducing pollutant loads in stormwater is to prevent harmful pollutants from being washed or dumped into the storm sewer system in the first place. MS4 permit holders, including the Town of Williston, must develop and implement a stormwater management program. Vermont issued a revised MS4 in late 2012 which, in addition to the stormwater management program, requires the development and implementation of flow restoration plans (FRP) for streams designated as being impaired by stormwater runoff, including Allen Brook in Williston. These “big-picture” issues, along with an assortment of localized stormwater problems, ranging from the flooding that occurred in the spring of 2011 to on-going erosion along several town roads, together represent a need for significant investment in stormwater management by the Town.

This Town-wide Watershed Improvement Plan was developed to provide a strategic approach that allows the Town to meet stormwater regulatory requirements while also addressing other pressing water resource concerns throughout the Town in an efficient and targeted manner.

1.1. Project Background

As precipitation falls on an undisturbed, natural landscape and moves through the hydrologic cycle, it flows through a complex system of vegetation, soil, groundwater, and surface waters. Natural events have shaped

Impaired Waters	
<u>Allen Brook</u>	
Pollutants of Concern:	<i>E. Coli</i> Stormwater Volume
<u>Muddy Brook</u>	
Pollutants of Concern:	Toxics Nutrients Temperature
<u>Lake Champlain</u>	
Pollutants of Concern:	Mercury & PCBs (in fish tissue) Phosphorus

these components over time to create a system that can efficiently handle stormwater through evaporation, transpiration, infiltration, and runoff. Alterations to the landscape change the way it responds to precipitation events. Management of land use, rainfall, stormwater management, and surface water (i.e. stream and lakes) are interrelated, and the management practices chosen all influence water quality and stream health.

Watersheds are interconnected networks of streams and watercourses in which a change at any location can carry throughout the system. Many factors influence exactly how stormwater runoff from a particular site will affect the other parts of the watershed, but this impact can be significant relative to other sources of pollution. Stormwater runoff affects water quality, water quantity, habitat and biological resources, public health, and the aesthetic appearance of the receiving water. Stormwater controls, in contrast, are typically conceived and implemented on a project-by-project basis. These projects are analyzed for their individual stormwater impacts, and not in the context of their impact on an interconnected hydrologic and hydraulic system. It is well documented, however, that the cumulative effects of individual land surface changes dramatically influence flooding conditions and contribute to degradation of water quality (NRC, 2009).

What is a watershed?

A **watershed** is any area of land in which all water runoff from its surface flows to the same drainage point. Watersheds are sometimes referred to as drainage areas.

Watersheds are important because they are the basic unit of analysis for all surface water management. They come in all shapes and sizes, and are defined based on the intended study area.

Watershed management practices have direct impacts on water quality in local creeks and streams (e.g., Allen Brook, Muddy Brook), as well as downstream waterbodies (e.g., Winooski River, Lake Champlain). Any decisions that affect land use have stormwater management ramifications and, in turn, impact all downstream water resources. The findings of one recent study (Troy et al., 2007) suggest that “land-use changes in the Basin have increased phosphorus levels in Lake Champlain, especially conversion of agricultural areas and forests to developed uses.”

Vermont’s streams, rivers, and Lake Champlain are vital economic resources. As such, the quality of local receiving waters affects both economic interests and quality of life in the surrounding areas. In Chittenden County, the local economy depends, in part, on the revenue gained from outdoor activities enjoyed in and on the water. Protecting the quality of surface waters is one of the most important commitments communities can make to protect the economic interests of residents.

Taken together, these elements emphasize the need for a holistic planning effort that considers the interconnected nature of land use, stormwater management, and river management in order to achieve overall watershed goals.

1.2. Goals of this Plan

The Town of Williston has three over-arching goals with regard to its surface water resources:

- Restoring the health of Allen Brook and Muddy Brook, to the point where these waters can be removed from the state’s 303(d) list of impaired waters;
- Preventing the addition of other local streams to the list of impaired waters; and

- Promoting healthy populations of fish and other aquatic organisms in all of the Town’s waters (Town of Williston, 2010, Chapter 29).

A variety of approaches will be taken to achieve these goals. For Allen Brook, which is considered to be stormwater impaired, the approach will include development of a Flow Restoration Plan (FRP), as required by the MS4 permit. For Williston’s other receiving streams, including Muddy Brook, Sucker Brook and the Winooski River, the approach will be supported and informed by the development of this Town-Wide Watershed Improvement Plan.

The purpose of a FRP is to identify stormwater treatment practices (including retrofits to existing practices) that will be recommended for implementation in an effort to achieve established Total Maximum Daily Load (TMDL) flow targets, and eventually to allow the impacted stream to be removed from the state’s list of impaired waters. Because the “pollutant of concern” in waters designated as stormwater-impaired is stormwater runoff volume, the practices identified in a FRP are specifically targeted towards reducing peak stormwater flows that enter a stream.

This Town-Wide Watershed Improvement Plan similarly identifies stormwater treatment practices and retrofits that, if implemented, can result in improved stream quality. Although there are numerous stressors in any watershed – ranging from bacterial contamination to the presence of toxics to excess nutrients – the focus of this plan is identifying areas of active erosion or other sources of sediment in the stream’s watershed, and developing appropriate priorities and recommendations for addressing these sources.

1.3. Project Overview

In order to achieve the most effective and efficient solutions, issues related to land use, surface water quality, and stormwater management should not be viewed independently. Rather, localized stormwater problems need to be examined in a regional context to determine their relative contribution to the overall condition of the watershed. This Town-Wide Watershed Improvement Plan is responsive to the existing characteristics of the watershed, connecting land use, stormwater management, floodplain management, river corridor management, and public infrastructure needs to more effectively address all of the issues which contribute to water quality. This will enable the Town to more efficiently plan for and implement watershed management activities.

The overall objective of this project and resulting Phase 1 report is to provide the Town of Williston with a strategic approach for meeting stormwater regulatory requirements while addressing pressing water resource concerns in an efficient and targeted manner. Specific project goals include:

- Incorporating information from existing plans and datasets to create a single, town-specific resource to guide future stormwater management activities
- Providing a means for comparing information about different watersheds
- Developing recommendations to address stormwater problems, including:
 - A prioritized list of problem areas that can assist stakeholders in directing resources to high priority projects; and
 - Potential revisions to town regulations or ordinances needed to encourage location-specific management activities.

2. GENERAL DESCRIPTION OF THE STUDY AREAS

The Town of Williston, located in Chittenden County, had a population of 8,698 at the 2010 Census, an increase of over 1,000 people since the 2000 census (U.S. Census Bureau, 2011). The Town covers a total area of more than 30 square miles, although most of the residential and commercial development is concentrated along Vermont Routes 2 and 2A. Since this document is focused on water resource concerns and watersheds, rather than political boundaries or transportation corridors, are the basic unit of analysis for planning. The Town of Williston lies wholly within the Lake Champlain basin. Runoff from the town eventually finds its way to the lake by one of several paths, most notably the Winooski River, which are shown on Map 1 in Appendix A. Each of the Town's watersheds is described in more detail below.

2.1. Allen Brook

Allen Brook originates above Mud Pond and flows northwest to join Muddy Brook just before that stream's confluence with the Winooski River. The Allen Brook watershed is Williston's largest, encompassing roughly 10.8 square miles (6,900 acres), or about one-third of the town (Town of Williston, 2011). The main stem of Allen Brook is approximately 11 miles long from its headwaters in the Sunset Hill area of Williston to its confluence with Muddy Brook along River Cove Rd.

Allen Brook is designated as a Class B waterway by the State of Vermont, meaning that the brook is suitable for "aquatic habitat, boating, swimming and public water supply with filtration and disinfection." Since 1992, the section of the Allen Brook upstream of Industrial Avenue has been included on the *Vermont 303(d) List of Waters* as impaired for stormwater and *E. coli* (Vermont DEC, 2012).

2.2. Muddy Brook

Muddy Brook runs along Williston's western border, forming the town's boundary with South Burlington. Approximately 9.8 square miles (6,300 acres) of this 20.8-square mile watershed is in Williston. The headwaters of Muddy Brook are found upslope of Shelburne Pond in the towns of Shelburne and South Burlington. Shelburne Pond is the dominant surface water feature in the upper watershed. It has an area of 452 acres and is fed by the mainstem of Muddy Brook, as well as other smaller tributaries. The Muddy Brook watershed also includes the Sucker Brook watershed (Section 2.3). In addition, the watershed includes the commercial and retail development in Taft Corners and Maple Tree Place. Muddy Brook is listed on the *Vermont 303(d) List of Waters* as impaired for toxins, nutrients, and temperature.

2.3. Sucker Brook

Sucker Brook is the major tributary to Muddy Brook and spans portions of the towns of Williston and St. George. The brook drains an area of approximately 7.4 square miles, 3 square miles (1,920 acres) of which is in Williston. Land cover in the watershed is predominantly second-growth forest and agriculture with some areas of low density residential development. In the 1980s, a severe rainstorm forced a tributary of the Sucker Brook to change its course, abandoning a 30-foot waterfall and flowing instead into a nearby gravel pit located on town land just off Route 2A. Over time, the gravel pit eroded, forming a large unstable canyon and sending substantial volumes of sediment downstream. From 2003-2007, the Town conducted a multi-phase stream stabilization and restoration project along the eroded canyon to successfully address the erosion.

2.4. Winooski River

The Winooski River has its source in the northeast corner of Washington County in the town of Cabot, then courses northwesterly for approximately 90 miles before flowing into Lake Champlain just north of Burlington. Its drainage area of about 1,080 square miles covers 11.9 percent of Vermont. The basin occupies all of Washington County, a little less than half of Chittenden County and small parts of Lamoille and Orange Counties (VTANR, 2012). In Williston, several small tributaries flow directly into the Winooski River. Cumulatively, the watersheds of these streams cover about 8.4 square miles (5,400 acres) of the town (Town of Williston, 2011).

2.5. Other Watersheds in Williston

Watersheds that are located partially within the Town of Williston, but which were not within the scope or focus of this project, include Lake Iroquois, Patrick Brook, and Johnnie Brook. Each of these watersheds is described briefly below.

2.5.1. Lake Iroquois & Patrick Brook

Lake Iroquois and the surrounding lands are part of the LaPlatte River watershed. The lake drains into Patrick Brook, which joins the LaPlatte River in Hinesburg. The river then flows into Shelburne Bay. This watershed includes about 1.7 square miles (1,100 acres) in Williston (Town of Williston, 2011).

The Vermont Agency of Natural Resources has classified Lake Iroquois as eutrophic, and the elevated nutrient levels in the Lake have contributed to the spread of the invasive aquatic plant, Eurasian Watermilfoil (*Myriophyllum spicatum*) (Town of Williston, 2011).

2.5.2. Johnnie Brook

The Town of Williston also includes less than one hundred acres of the Johnnie Brook (Town of Williston, 2011). The portion of the Johnnie Brook watershed in Williston, along the town's eastern border south of I-89, is mostly wooded with low-density residential development.

3. EXISTING PLANS AND DATA

Numerous and varied groups and individuals have invested considerable effort in evaluating different components of Williston's water resources, and the important interface between water resources and local land use decisions. Some evaluations have followed watershed boundaries, while others have followed political boundaries. The following sections identify evaluations completed to date, with emphasis on work most relevant to the Town of Williston, and most relevant to future efforts to develop a list of strategic, prioritized projects that could be undertaken to improve water quality in and around Williston.

3.1. Watershed-Based Assessments

The assessments described below are generally led by the Agency of Natural Resources. These include:

- Basin planning, completed primarily to guide VTANR in its own work and in collaborative projects with the public, municipalities, and other State and federal agencies. The basin plans have a five-year scope. The recently completed *Winooski River Basin Water Quality Management Plan* (VTANR, 2012) covers more than 90% of the Town of Williston's land area.
- Stream geomorphic assessments (SGA), undertaken to understand the natural tendencies of a particular reach of stream or river, its current condition, and what changes may be anticipated in the future. Stream geomorphic assessments have been completed for many of the stream/river segments within the Town of Williston, including: Allen Brook, Muddy Brook including Sucker Brook, the Winooski River, and Patrick Brook. Each of these studies is described in more detail in the sections that follow.
- Water quality monitoring, initiated to monitor the water quality of Williston's streams and use the data to inform mitigation efforts. In 2007, the Town was awarded a Vermont DEC LaRosa Partnership grant to begin baseline water quality monitoring in Allen Brook. The monitoring parameters originally included nitrogen, phosphorous, and E. coli, and chloride and turbidity were added in 2010. The monitoring and results to date are described below, where monitoring has been conducted.
- TMDL development, to establish the maximum amount of a pollutant (e.g., bacteria, nutrients, excess stormwater flows) that a waterbody can assimilate and still meet state-established water quality standards. They are based on the relationship between pollution sources and in-stream water quality conditions. A TMDL addresses a single pollutant or stressor for each waterbody. TMDLs have been developed and approved for bacteria (as part of the *Vermont Statewide TMDL for Bacteria-Impaired Waters* (VTANR, 2011)) and stormwater for Allen Brook. The previously approved Lake Champlain phosphorus TMDL is currently under review by EPA Region 1.

3.1.1. Allen Brook

A substantial number of assessments, including all four of the types of assessments described above, have been completed or are in progress for Allen Brook and its watershed. The work products resulting from these efforts are briefly summarized below.

3.1.1.1. Winooski River Basin Water Quality Management Plan (2012)

VTANR published the water quality management plan for the Winooski River basin in 2012. This report provides an overview of the Winooski River Basin's surface waters, including Allen Brook, and a description of ongoing and future steps to restore and protect those waters. The plan presents the recommendations of local watershed residents, stakeholders with varying interests, VTANR and professionals from other State and federal agencies that will guide efforts to improve both water quality and aquatic habitat, in the basin over the next five years. It includes several statements specific to Allen Brook, including:

- Identifying the implementation of efforts to address Bacterial TMDL for Allen Brook is one of the top ten priority strategies in the management plan. Specific measures that are likely to be incorporated in VTANR's implementation plan to meet the Bacteria TMDL include:
 - Identifying agricultural operations in the watershed and working with agricultural resource staff to identify and address pathogen sources;
 - Identifying and implementing stormwater management opportunities that reduce bacterial loads to receiving streams; and
 - Developing and implementing a pet waste management plan with the Town and other landowners.
- Increasing the establishment and enhancement of woody riparian corridors on stable reaches of Allen Brook as a priority strategy for enhancing stream stability; and
- Supporting the implementation of this Town-Wide Watershed Improvement Plan, once complete.

3.1.1.2. Flow Restoration Plan (2012)

A draft flow restoration plan (FRP) was recently completed as part of an effort to implement the stormwater TMDL for Allen Brook (DuBois and King, 2012). The goal of the FRP was to identify BMPs that will achieve flow targets set forth in the 2008 TMDL. The plan identifies expired stormwater discharge permits and opportunities for implementation of BMPs within town boundaries. The two specific opportunities for BMP implementation on town-owned land that were identified in the FRP are:

- A parcel south of and behind the Town offices located off of U.S. Route 2. This parcel is currently undeveloped meadow which is occasionally hayed. The BMP could be installed near the south side of the parcel where it abuts I-89, and thus capture drainage from the wooded areas to the south of the highway.
- A parcel between U.S Route 2 and Interstate I-89. This parcel is currently undeveloped; portions are utilized as farmland. The BMP could be installed near the south side of the parcel where it abuts I-89, and thus capture drainage from Hurricane Lane and the wooded areas to the south.

The FRP also identified three opportunities within VTrans right-of-way for additional stormwater management. In addition to identifying these opportunities, a construction schedule is generally discussed, and construction cost estimates are provided for individual BMP projects. The municipal separate storm sewer system (MS4 permit), issued on December 5, 2012, requires the development and implementation of a FRP for each of Vermont's stormwater impaired watersheds (Vermont DEC, 2012a).

3.1.1.3. USGS Stream Gage (2007-present)

Since October 2007, the U.S. Geologic Survey (USGS) has operated a flow monitoring station in cooperation with the Vermont Agency of Transportation and the Town of Williston on Allen Brook where it passes under Route 2A. The data are available on-line at: <http://waterdata.usgs.gov/vt/nwis/uv?04290335>.

3.1.1.4. Water Quality Monitoring (2007-present)

The Williston Conservation Commission (WCC) has collected water quality samples along the Allen Brook since 2007 (no samples were taken in 2009). The parameters sampled included Total Nitrogen (TN), Total Phosphorus (TP), *E. coli*, Chloride (Cl), and Turbidity (NTU). Three years of data are available for TN, TP, and *E. coli* (for most sites), although not all parameters were sampled every year. Key findings from the sampling effort include:

- TN and TP concentrations were greatest in the upstream reach of the Allen Brook and lowest in the downstream reach. These results were somewhat expected because agricultural land is concentrated in the upstream reach.
 - TP levels ranged from 0.021 to 0.157 mg/L
 - TN levels ranged from 0.31 to 1.05 mg/L
- Concentrations of *E. coli* were highly variable over the sampling period. There appeared to be no strong correlation between *E. coli* spikes and rain events.
- Turbidity levels were generally greater downstream than upstream, though no trends or explanations were apparent.
 - Turbidity levels ranged from 0.79 to 17.20 NTU
- Chloride concentrations generally increased as samples were taken further downstream.
 - Chloride levels ranged from 20.5 to 150.4 mg/L
- Allen Brook exceeded the water quality standards for *E. coli* and turbidity in the majority of samples collected; Allen Brook did not exceed the federal standards for Cl. Numerical standards do not yet exist for TN and TP levels in streams and rivers, but TP concentrations observed are generally above the in-lake standard (0.010 mg/L) established for phosphorus in segment of Lake Champlain to which Allen Brook ultimately discharges.

3.1.1.5. Allen Brook Restoration Project (2011)

Stream geomorphic studies of Allen Brook (see Section 3.1.1.7 and 3.1.1.9, below) found that a number of reaches of Allen Brook lack forested riparian buffers. Inadequate buffers have led to streambank failure and incision, which in turn has caused devastating habitat loss for fish and aquatic insects. The Town is committed to acquiring conservation easements and reforesting riparian buffer corridors along impaired portions of the Allen Brook and its tributaries, but needed a systematic approach for prioritizing the planning, design, coordination, and implementation of permanently conserved vegetated riparian buffer areas along segments of the Allen Brook and its tributaries.

The *Allen Brook Restoration Project* (KAS, 2011) identified 158 parcels that contain property within the 150' buffer corridor along the impaired portion of the Allen Brook and its tributaries. Each parcel was ranked based on suitability, need, and feasibility for restoration within the 2011 calendar year; a “short list” of 30 parcels was identified and cross-checked against the list of project sites contained within the Allen Brook Watershed Departure Analysis and Project Identification Summary (see section 3.1.1.7, below). In general, parcels on the short list tended to have larger riparian buffer areas (over an acre), incised stream banks, poor riparian buffer quality, and erodible soils. The Town has used the short list of parcels from this report to guide its conservation efforts in the Allen Brook watershed.

To date, the Town has permanently conserved approximately 25-acres of stream buffer (ranging in width from 50 to 150 feet) and re-vegetated 18 acres along the banks of the Allen Brook (Town of Williston, 2011).

3.1.1.6. Biological and Aquatic Life Use Attainment Assessment of Allen Brook (2011)

VT DEC uses standard protocols for periodically assessing the biological condition of stream sites across Vermont, including segments of Allen Brook, Muddy Brook, and Sucker Brook. An in-depth evaluation of the biological data was completed for Allen Brook following the 2010 monitoring season (VT DEC 2011). The report includes a number of interesting findings, such as:

- Habitat observations show a slight trend in decreased canopy cover, moving from upstream to downstream;
- A number of freshwater mussel species, including two uncommon species, are present in Allen Brook;
- It is likely, given the level of activity in the watershed, as well as the complexity and diversity of the landscape in the Allen Brook watershed, that factors other than “sediment” (e.g., nutrients and hydrology) contribute to the impacts observed within the biological community.

3.1.1.7. TMDL to Address Biological Impairment (2008)

Allen Brook is listed as impaired because of diminished biological integrity due to “excessive stormwater flows.” A TMDL for excess stormwater in Allen Brook has been developed by VT DEC and subsequently approved by EPA Region 1 (VT DEC, 2008). Because the “pollutant of concern” is stormwater runoff volume, the loading capacity is the greatest volume of stormwater runoff Allen Brook can receive without violating the stream’s aquatic life criteria. Background and reasoning behind the impaired designation is discussed as well as TMDL development.

3.1.1.8. Watershed Departure Analysis and Project Identification Summary (2008)

The watershed departure analysis presents the findings of a study of the sediment source and transport characteristics in Allen Brook (Fitzgerald, 2008). Results indicated that impervious cover, man-made drainage infrastructure, and loss of wetlands have impacted the geomorphology of the Allen Brook, leading to increased erosion rates and sediment transport capacity. A prioritized list of 21 watershed improvement projects was provided as part of the report, with 12 restoration projects identified as high or medium priority. These projects were further characterized as “passive” (generally conservation based and do not require further study to pursue implementation) or “active” (projects which will require further study prior to implementation).

A stream reach map for Allen Brook is included as Map 2 in Appendix A. Projects ready to pursue implementation (passive restoration), by stream reach and project number, included:

1. M01-1: Develop conservation easements for parcels on lower and middle reach. Ideally completed in conjunction with buffer planting project described in project M01-2 (high priority).
2. M02-1: Develop conservation easements for parcels on lower section of reach where lower sloped land adjacent stream channel could face development pressure in future (medium priority).
3. M03-A-1: Develop conservation easements for parcels on west bank parcels in middle and upper segment where adjacent land could face development pressure in future (medium priority).
4. M03-C-1: Develop conservation easements for parcels on lower and middle segment, on the left bank. Ideally completed in conjunction with buffer planting project described in project M03-C-2 (high priority).
5. M05-B-1: In areas where corridor is not developed, pursue conservation easements for parcels in upper reach in between Route 2 and I-89 (high priority).

Projects requiring further study (active restoration), by stream reach and project number, included:

1. M01-2: Plant buffer with native woody vegetation in the middle and lower reach (high priority).
2. M01-3: Replace bridges with appropriately sized structures. Follow new River Management Program guidelines to accommodate 100% of equilibrium channel width (medium priority). Additional information is provided on Problem Area Data Sheet AB-03, presented in Appendix B.1.
3. M03-C-2: Plant buffers in middle-lower segment where no woody vegetation exists on either side, and in upper segment on left bank where there is limited woody vegetation. Other active restoration approaches for this segment, such as restoration of channel geometry, are discouraged in the short term due to the current state of channel adjustment (high incision; stage II CEM; medium priority).
4. M04-A-1: Plant stream buffer in areas above Old Stage Rd where bank vegetation is lacking (medium priority). Additional information on this need is provided on Problem Area Data Sheet AB-46, presented in Appendix B.1.
5. M04-A-2: Investigate feasibility of active channel restoration of bedforms and floodplain features in upper segment (medium priority). Additional information is provided on Problem Area Data Sheet AB-47, presented in Appendix B.1.
6. M05-B-2: Plant stream buffers in vicinity of Route 2 crossing. Stream boundaries lack native woody vegetation above and below crossing for ~300' on both sides (medium priority).
7. M07-1: Plant stream buffer, and install fencing to exclude grazing animals from stream channel in lower reach (high priority). Additional information is provided on Problem Area Data Sheet AB-50, presented in Appendix B.1.

3.1.1.9. Chloride Assessment of Select Urban Streams in Chittenden County (2007)

Chloride concentrations were measured in six streams in the greater Burlington area, including Allen Brook and a tributary to Muddy Brook, and evaluated in an effort to determine the extent to which chloride levels in the streams could be contributing to observed impacts to aquatic life (VT DEC, 2007). During the study period, Allen Brook never exceeded the EPA's chronic criterion for chloride; the tributary to Muddy Brook, however, exceeded the criterion 66% of the time. The study concluded that chloride is likely contributing to stormwater-driven impacts in the tributary to Muddy Brook, but is not likely to be significantly contributing to the biological impacts observed in Allen Brook.

3.1.1.10. Stream Geomorphic Assessment (2005)

Stream geomorphic assessment data specific to Allen Brook was collected in 2005. This work confirmed the significance of in-stream sediment generation, as opposed to production zone sediment inputs, and its resultant negative impact on aquatic biota and habitat (Fitzgerald, 2005). Results from the geomorphic assessment indicate that the stream channel is highly unstable and that the potential for more degradation is high. Of 15 reaches assessed in the Allen Brook watershed, one was rated as being in "poor" geomorphic condition, 11 were rated as being in "fair" condition and 3 were rated as "good". In the same 15 reaches, sensitivity to further channel instability was rated as "very high" in 11 reaches, "high" in three reaches and "moderate" in the remaining reach. These conditions reflect a generally degraded aquatic habitat, where 10 reaches were rated as having "fair" habitat conditions with the remaining five were rated as "good".

3.1.1.11. Watershed Improvement Plan and Recommendations for a TMDL for Sediment (2003)

Prior to the development of the stormwater TMDL for Allen Brook, which was approved by EPA in 2008, VTDEC sponsored an effort to develop a watershed restoration plan for Allen Brook and to make recommendations for the development of a sediment-based TMDL (Barg et al., 2003). The objective of the plan was to develop an alternative approach to restore Allen Brook, and more generally determine a cost-effective means of restoring waters impaired by non-point sources.

The authors of the plan collected substantial scientific data and completed a variety of assessments on the Allen Brook watershed. These datasets included fluvial geomorphology characteristics, stream biology indicators, and water quality parameters. The team also performed watershed analyses including calculations of watershed impervious coverage and modeling of sediment loads in the watershed.

The information collected and compiled for this report was analyzed and used to provide specific recommendations for management activities that should be implemented to address existing impairments and prevent future degradation of the watershed. The report provides recommendations on areas that may be addressed through the development of ordinances and incentives to prevent further degradation of Allen Brook. Groups of recommendations in the report include:

- Stormwater Management and Retrofit Opportunities
- Prediction of stream adjustment based on land use
- Adopting Management and Maintenance Suggestions
- Changes to local ordinances

- Pollution prevention
- Developing a stormwater utility
- Best Management Practices During Construction
- Better Site Design
- Road Management
- Best Management Practices (BMPs)
- Prevention (via application of non-structural stormwater credits for new projects)

Detailed recommendations for retrofit opportunities are included with priority rankings for over 100 stormwater management systems in the watershed. At the time this report was finalized, the highest-priority retrofit opportunities included the following areas and issues, which are quoted directly from the report:

1. Meadow Ridge subdivision. Uncontrolled runoff from the subdivision overtops South Road and has washed manure from the Siple farm manure pit into Allen Brook. This has contributed to high nutrient loading to Allen Brook that is evident by the presence of long strands of attached filamentous algae in Reach 9. A stormwater detention pond was never built as required by the VTDEC Stormwater Discharge Permit. Swales were not dimensioned as shown in the permit and are conveyance ditches instead that are a source of sediment. Elements of this need are also documented Problem Area Data Sheets AB-50 and AB-52, presented in Appendix B.1.
2. South Ridge subdivision. The stream inventory of Allen Brook showed it to be in poor condition immediately downstream of this subdivision. While there are two other subdivisions in the general area, South Ridge is the largest, closest to the brook, and contains the most impervious area. Existing stormwater ponds need maintenance and retrofitting. Elements of this need are also documented Problem Area Data Sheet AB-48, presented in Appendix B.1.
3. Taft's Farm subdivision. This subdivision straddles Allen Brook with very little buffer left along the stream corridor. Significant erosion from some storm drain outfalls exists, treatment at certain discharge points to Allen Brook is insufficient or nonexistent, and basins are in need of maintenance and retrofitting. Various elements of this need are also documented Problem Area Data Sheets AB-16, AB-17, AB-18, AB-19, AB-20 and AB-21, presented in Appendix B.1.
4. Williston Hills subdivision. There was extensive gully erosion below the culvert outfall for the storm drain collection system serving this area.
 - In 2006, the Winooski Natural Resources Conservation District began efforts to stabilize the gullies. Construction for Gullies A and B occurred in the fall of 2007. Construction for the largest of the three gullies, Gully C, was completed in the fall of 2008. In November 2008, the site was re-vegetated with 185 trees and more than 400 shrubs from local nurseries.
5. Avenue D (Whitcomb Industrial Park). Most lots were not required to have a stormwater discharge permit since provisions to infiltrate stormwater on-site were included in construction plans. Many of the proposed infiltration STPs were never built or are not functioning as intended.

There is gully erosion from both permitted and non-permitted discharge points. Groundwater impacts are a concern since this site was once a sand pit and has soils with high infiltration rates. This need is also documented Problem Area Data Sheets AB-26 and AB-29, presented in Appendix B.1.

3.1.2. Muddy Brook

Geomorphic assessments and water quality monitoring efforts have been completed or are underway in the Muddy Brook watershed. The work products resulting from these efforts are briefly summarized below.

3.1.2.1. Water Quality Monitoring (2012-present)

The Chittenden County Stream Team initiated a volunteer-led water quality monitoring effort in six Chittenden County streams in the summer of 2012, including Muddy Brook. Samples were collected twice monthly at three sites along Muddy Brook (one in Williston and two in South Burlington), and analyzed for phosphorus, nitrogen, turbidity and chloride levels. This data is being made available on-line on the Stream Team website (CCST, 2013), and shows:

- Total phosphorus levels ranging from 0.043 to 0.266 mg/L
- Total nitrogen levels ranging from 0.70 to 2.75 mg/L
- Turbidity levels ranging from 4.81 to 53.7 NTU
- Chloride levels ranging from 21.3 to 143 mg/L

Although it is impossible to make direct comparisons between sediment and nutrient loads in Allen and Muddy Brooks based on this data, the data do suggest that nutrient concentrations and turbidity levels are generally higher in Muddy Brook than in Allen Brook (see Section 3.1.1.4); chloride levels are comparable in both brooks.

3.1.2.2. Phase 1 and 2 Stream Geomorphic Assessment Summary (2009)

This report provides the results from Phase 1 and 2 stream geomorphic assessments of Muddy Brook (Fitzgerald and Parker, 2009). The two goals of the report are:

- Assessing the current geomorphic stability and habitat conditions in the watershed to compliment VTDEC biological sampling data, and
- Establishing baseline data for long-term monitoring purposes.

The nature of the watershed was characterized. The extent of departure from reference conditions was reported, along with suspected causative factors; 16 bridges and culverts were specifically assessed for their impact on channel equilibrium conditions. A stream reach map for Muddy Brook is provided in Appendix A. Reach-specific recommendations for restoration activities from this work are presented below without modification:

- Arrest Headcuts: The two headcuts noted on Tributary 3 are causing the export of large amounts of sediment to the downstream reaches. The headcut located downstream of Harvest Lane on

T3.01-E has the potential to impact the road crossing in the near future (5-10 years), and will need to be addressed with an active channel management approach.

- Continuous monitoring of both headcut sites is recommended to determine the rates of migration and the immediacy of restoration needs.
- Corridor Protection: Protecting the stream corridor through conservation easements is recommended along reaches of the lower and middle watershed that are susceptible to future development. This approach is especially important in reaches or segments that are in vertical adjustment: M02, M10, T3.01-E.
- Undersized Stream Crossings: Severely undersized culverts are causing channel adjustments in the following reaches: M02, M08, M13, T2.01-A, T3.02-A. As noted in the structures summary, these structures should be considered high priority for replacement by town and state agencies.
- Derelict Stream Crossing: There is an inactive stream crossing in Reach M09. The abutments act as a channel constriction and are causing erosion and bifurcation of the channel. Given that the stream crossing is no longer in use, removal of the abutments is recommended.

The Problem Area Data Sheets for Muddy Brook, presented in Appendix B.2, indicate which problem areas were initially identified by reviewing the Phase 1 and 2 SGAs. In total, nine of the 19 stormwater problem areas documented in the Muddy Brook watershed were drawn from the SGAs; these are: MB-02, MB-06, MB-07, MB-08, MB-09, MB-11, MB-14, MB-15, and MB-25.

3.1.3. Sucker Brook

Only Phase 1 and 2 Stream Geomorphic Assessments have been completed for Sucker Brook.

3.1.3.1. Phase 1 and 2 Stream Geomorphic Assessment Summary (2007)

This report provides the results from Phase 1 and 2 stream geomorphic assessments of Sucker Brook (Fitzgerald, 2007). The intent of the document is to:

- Concisely summarize Sucker Brook watershed zones and geomorphic processes; and
- Highlight important or extraordinary information for those using the data.

The nature of each section of the watershed is characterized and each reach described. The report concluded that the Sucker Brook watershed supports many sections in good to reference conditions – especially in steeper terrain. The lower reaches are more impacted due to a lack of riparian buffers and channel straightening associated with agricultural use. Stream reach maps for Sucker Brook are presented in Appendix A. Potential restoration projects identified during this work are listed and briefly described below by stream reach:

- Route 2A Valley (M04 to M07; T1.01):
 - Protection of the corridor from future encroachment
 - Replacement or retrofit of several undersized culverts to accommodate the large volume of sediment working its way through the reach, and the resulting vertical (aggradation followed by incision) and lateral (planform change) adjustments that will continue to occur

over the next 10 to 20 years. The severe aggradation of coarse material in reach T1.01 has been problematic for the Town of Williston and the landowners whose driveways and roads cross the channel. This need is also documented Problem Area Data Sheet SB-01, presented in Appendix B.3.

- North Branch Middle Zone (T1.02 to T1.04-A):
 - An incised tributary entering from the north was noted in Segment T1.04-A. This tributary enters the north branch channel west of Lyman Road and originates from an area of recent residential development on the north side of Old Creamery Road. Recently built housing on Highlands Drive and Overlake View, and an increase in upslope impervious cover, may be resulting in excess hydraulic loading and channel incision. This need is also documented Problem Area Data Sheet SB-09, presented in Appendix B.3.
- Old Creamery Road Zone (T1.04-B to T1.05-C)
 - In-stream structures (e.g., weirs) in reaches T1.04-B and T1.05-A are permanent enough to remain in place during moderate flow events (below bankfull) when fine sediment is being transported through the channel. This is resulting in aggradation upstream of the structures that is degrading the habitat. It is recommended that these structures be removed to reduce further habitat impacts.
 - Additional projects in this densely residential zone would include the re-vegetation of the buffers with native woody vegetation.
 - Future efforts to identify specific projects in this watershed zone would involve extensive public outreach, given the large number of residential properties found along the channel, especially in segment T1.05-A.

3.1.4. Winooski River

A substantial number of assessments have been completed for the Winooski River watershed. The work products resulting from these efforts are briefly summarized below.

3.1.4.1. Winooski River Basin Water Quality Management Plan (2012)

As discussed in Section 3.1.1.1, this report gives an overview of the waters of Winooski River watershed, and describes present and future measures for restoration and protection of these water resources (VTANR, 2012). The ten highest-priority strategies for improvement initiatives are presented and discussed. High-priority strategies that are generally relevant to the Town of Williston include:

- Identify culvert replacement projects in the basin that will improve geomorphic stability of the stream as well as improve fish passage.
- Promote agricultural programs in targeted areas that incentivize fencing, buffers, grassed waterways, barnyard treatments, conservation tillage practices, and cover cropping.
- Work with towns to protect river corridors and promote flood resiliency by establishing Fluvial Erosion Hazard zones and buffer zones in local zoning.

- Identify wetlands on agricultural lands for phosphorus retention, and in the river corridor for sediment attenuation, prioritize and conserve and/or restore.
- Hold an annual Vermont Invasive Patrollers (VIP) training to support the establishment of VIP programs for lakes and ponds in the basin.
- Encourage use of rivers and lakes in the basin to increase people's appreciation.

3.1.4.2. Mid-Winooski Watershed Phase I Stream Geomorphic Assessment (2007)

This report provides a broad assessment of geomorphic conditions in the Winooski River from Montpelier downstream to Alder Brook (Bear Creek Environmental, 2007). The goal of the report is to provide a stream-impact ranking to each reach to help direct the focus of more detailed Phase 2 Geomorphic Assessments. Significant channel alterations and development within the watershed are cited as the major influences on the geomorphic conditions of this stretch of the Winooski River. Specific reaches of the Winooski and major tributaries are identified in fair to poor condition and are recommended for more detailed, Phase 2 Assessments. Due to high levels of observed impact and reach condition, all of the main stem reaches are recommended for a Phase 2 assessment, including Reach R07 on the Winooski River, located on Williston's northern border between Alder Brook and the I-89 and Route 2 bridges in Richmond.

3.1.4.3. Fluvial Geomorphology Assessment of the Lower Winooski River, Vermont (2006)

This report provides a geomorphological assessment of six identified reaches of the lower 21.7 miles of the Winooski River—from the confluence of Alder Brook to Lake Champlain (Field Geology Services, 2006). The report cites the natural constrictive effect of the Winooski Gorge, reduced sediment loads (due to dams and 20th century reforestation of the watershed), and minor channel straightening as being the primary factors inhibiting sediment-transport equilibrium. Recommendations for restoring channel stability and aquatic habitat are provided, though all of the substantial project identification in this report is for the reaches of the Winooski River downstream of Winooski Falls, well outside of the Town of Williston.

3.1.5. Other Watershed Assessments

An evaluation of potential stormwater problem areas in the Lake Iroquois, Patrick Brook and Johnnie Brook watersheds was not undertaken as part of this project. There are, however, several assessments that have been completed in these watersheds as described below.

3.1.5.1. Lake Iroquois

Several studies have been completed on Lake Iroquois since the summer of 2007 when the Lake Iroquois Association was formed. Of particular interest:

- In 2010, volunteers for the Lake Iroquois Association, with assistance from VT DEC, completed a three-phase survey (in-lake, shoreland, and watershed) to identify and prioritize required actions that make the biggest impact on improving the lake, though this plan has not yet been published (Lake Iroquois Association, 2012). This survey resulted in the identification of several stormwater mitigation projects.

- In 2010, the organization received a Better Backroads grant from VT ANR to study an ongoing storm runoff issue at Pine Shore Drive in Hinesburg and design solutions to mitigate flow of storm water into the lake, resulting in the construction of improvements in the summer of 2012 (Handler, 2012).
- In 2011, LIA received a VT ANR Ecosystem Restoration Grant to develop a storm water runoff mitigation solution for the area of Shadow Lane, also in Hinesburg (Lake Iroquois Association, 2012). A similar Ecosystem Restoration grant that will enable the Lake Iroquois Association (LIA) and the Lake Iroquois Recreation District (LIRD) to stop gullyng on the Lake Iroquois public beach and the resulting erosion of sediments into the lake was awarded in the 2012 funding cycle, with completion of a final design expected in the late summer of 2013.
- LIA participates actively in both the Lay Monitoring and Vermont Invasive Patroller (VIP) programs, both of which are implemented state-wide by the VT DEC. Volunteers measure water clarity and nutrient levels, and regularly traverse the lake looking for new invasive aquatic plants and animals. In 2011, LIA implemented a sampling plan at several tributaries to determine the level of nutrients entering the lake. This effort is supported by a grant to cover the cost of chemical analysis at the state's LaRosa Lab.

3.1.5.2. Patrick Brook

Phase 1 and 2 geomorphic assessments have been completed for the LaPlatte River and its major tributaries including Patrick Brook (LaPlatte Watershed Partnership, 2006). Each of three main reaches in Williston was characterized as follows:

- Habitat and geomorphic conditions were assessed;
- Channel alterations, obstructions, and constructions were noted;
- Extraneous influences were documented; and
- Channel evolution stage was documented.

Channel alteration and channel constrictions were found to be major factors affecting the geomorphology of the reaches studied. Reaches that were altered are expected to be the most sensitive to future development. Guidance for future work is provided, though no potential restoration projects were identified in the Patrick Brook watershed up-stream of Lake Iroquois.

3.1.5.3. Lake Champlain

In early 2011, EPA withdrew their 2002 approval of the Vermont portion of the Lake Champlain TMDL for phosphorus. In reversing their decision, EPA noted that two elements of the TMDL did not comply with EPA regulations and guidance. Specifically, EPA found that the TMDL did not provide an adequate "margin of safety" to account for uncertainty in the true capacity of the lake to accommodate phosphorus pollution, and did not offer "reasonable assurances" that the called-for reductions in non-point source pollution would actually be achieved.

EPA has specifically noted that the 2002 TMDL allowed most of Vermont’s wastewater treatment facilities to have effluent phosphorus concentrations “well above levels that would otherwise be required in the absence of nonpoint source load reductions”. This statement suggests that additional phosphorus removal requirements for wastewater treatment facilities are likely under consideration. In addition, EPA has indicated their intention to treat MS4s as “point sources” in the revised TMDL. Other changes that EPA may be contemplating are less clear, but could involve:

- Requiring more communities to obtain MS4 (municipal separate storm sewer system) permit coverage. Currently MS4 designations are confined to the more densely populated areas of Chittenden County, with Rutland City and Town and St. Albans City and Town being added with the new MS4 permit signed in December 2012.
- Expanding the use of “residual designation authority” to require larger developed tracts to install stormwater management systems and obtain permit coverage.
- Requiring agricultural operations to obtain additional permit coverage beyond the medium-farm or large-farm operation (MFO or LFO) permits currently issued by the Agency of Agriculture.

3.2. Town-Wide Assessments and Programs

3.2.1. Municipal Ordinances and Regulations

In Williston, town regulations require runoff and erosion control plans for development in which the cumulative land disturbance is greater than two acres, and for projects of any size that will disturb land within watershed protection buffers or on slopes greater than eight percent (Town of Williston 2010, Chapter 29, Sections 29.2-29.4). Projects that disturb more than ¼-acre but less than two acres are required to follow the *Low Risk Site Handbook for Erosion Prevention and Sediment Control*, published by VT DEC.

3.2.2. NPDES MS4 Program and Minimum Control Measures

Williston is designated as an MS4 under the National Pollutant Discharge Elimination System (NPDES) Phase II stormwater requirements of the Clean Water Act (Vermont DEC, 2012a). This MS4 designation requires Williston to file a five year Stormwater Management Program (SWMP) which responds to six “Minimum Control Measures” (or “MCM”):

- Public Education and Outreach on Stormwater Impacts (MCM 1)
- Public Involvement/Participation (MCM 2)
- Illicit Discharge Detection & Elimination (MCM 3)
- Construction Site Stormwater Runoff Control (MCM 4)
- Post-Construction Stormwater Management for New Development and Redevelopment (MCM 5)
- Pollution Prevention/Good Housekeeping for Municipal Operations (MCM 6)

In addition, the Town is obligated to file an annual report describing stormwater management activities completed during the preceding planning year.

3.2.3. Stormwater Management Practices Study

In 2004, Hoyle, Tanner & Associates completed a study intended to assist the Town of Williston in identifying appropriate stormwater treatments practices that could be used within the planned densely developed “downtown” area that are consistent with current State stormwater treatment requirements (HTA 2004). The report includes an extensive literature review and a detailed discussion of nine specific stormwater treatment technologies. The technologies are: infiltration trenches and basins; porous pavement; open channel systems; underground and perimeter sand filter systems; manufactured underground filtering systems; detention ponds; underground storage systems; chambered separation treatment practices; and, swirl concentration treatment practices.

4. STORMWATER PROBLEM AREAS

One of the goals of this Plan is to “develop a comprehensive list of stormwater problems.” To achieve this goal, a thorough effort was made to identify existing problem areas, and then to evaluate the existing conditions and potential solutions.

4.1. Identification of Problem Areas

The first task was to identify the location and nature of existing drainage problems, and where appropriate, to gather field data for further analysis. A variety of strategies were employed to identify potential problem areas, including:

- Holding a public meeting to describe the project and solicit suggestions of potential problem areas;
- Distributing a “Citizen Questionnaire” via the Town’s website to gather additional public input on problem areas (see Appendix B);
- Performing windshield surveys of strategic areas throughout Town during, or immediately following, rain events; and,
- Meeting with Town staff to review the draft list of stormwater problem areas and solicit input.

In total, more than 100 problem areas were identified and geo-located. The geographical location data were used to plot all of the problem areas on a single map (see Appendix A, Map 7). Mapping problem area locations in this manner allows the viewer to identify isolated problems, and make a visual assessment of which problems may be part of a larger systemic issue. Systemic problems may warrant consideration of additional stormwater management regulations, in addition to efforts to correct the existing problems.

4.2. Evaluation of Problem Areas

Working from the list of potential problem areas, the Consultant Team visited each potential problem area location to directly observe the site. When no stormwater problems were evident and/or it was obvious that a solution had been implemented, the site was not evaluated further. Where an unresolved problem was found,

photos were taken of any areas of active erosion, and observations about the source or cause were documented. Each problem area was also evaluated with respect to:

- Relative environmental impact on the nearest receiving water
- Frequency of occurrence/discharge
- Current condition of existing facilities and related infrastructure
- Stability of the existing problem
- Impact to public infrastructure
- How realistic it is to fix the problem
- Whether there were impacts beyond water quality (e.g., aquatic organism passage inhibited)
- Whether the problem was part of a larger or more systemic problem

Each problem area was then given a score, with the intent of generally assessing the severity of existing problems, removing low priority problem areas from the dataset, and providing general guidance on the relative order in which the problems should be addressed when considered town-wide. Scores were assigned as follows:

- Relative environmental impact on receiving water (e.g., proximity, location, amount of sediment being mobilized), with 1 being least impact and 3 being greatest impact;
- Frequency of occurrence, with 1 = rarely/extreme rain events, 2 = occasional/moderate rain events, and 3 = frequent/ nearly every rain event;
- Current condition of existing facilities and related infrastructure, with 1 = good to excellent; 2 = fair to good; 3 = poor;
- Urgency of the need to fix the problem, with 1 = problem is unlikely to get worse if left untreated; 2 = problem may get worse, but immediate action not necessary; 3 = problem is getting worse, immediate action is necessary.

The complete set of problem area data sheets developed as part of this project are provided as Appendix C of this report, and summaries of evaluation results are presented in Tables 1-4 below. Only sites that scored priority 5 or above were listed in the tables, and the sites are listed in order of priority.

All problem areas that received an evaluation score of 8 or higher – generally indicative of an on-going or significant erosion and/or drainage problem – will be advanced to Phase 2 of the *Williston Town-Wide Stormwater Master Plan* project, with priority given to projects that the Town could expect to assume a leadership role in addressing. Those that involve issues where it would be appropriate for the Town to assume a leadership role will be advanced as well. As described in Section 6.1, Phase 2 entails completing an engineering analysis of each of the critical problem areas. During this analysis, a site-specific physical feasibility assessments will be completed that defines contributing drainage area, water table elevation, soil conditions, existing infrastructure, and traffic and pedestrian flow in order to more fully evaluate anticipated water quality benefits and constructability at each priority problem area.

Table 1: Allen Brook Watershed Problem Area Evaluation

	Site ID	Relative Impact	Frequency	Current Conditions	Urgency	Priority ¹
	AB-19	3	2	3	2	10
	AB-09	2	2	3	2	9
	AB-11	3	2	2	2	9
	AB-13	2	3	2	2	9
	AB-18	2	2	3	2	9
	AB-20	2	2	3	2	9
	AB-43	2	2	3	2	9
	AB-44	2	2	3	2	9
	AB-45	3	2	2	2	9
High Priority	AB-01	3	2	1	2	8
	AB-05	1	3	2	2	8
	AB-07	2	2	2	2	8
	AB-17	2	2	2	2	8
	AB-24	2	2	2	2	8
	AB-26	1	2	3	2	8
	AB-29	2	2	2	2	8
	AB-30	2	2	3	1	8
	AB-32	1	3	2	2	8
	AB-34	3	1	2	2	8
	AB-39	2	2	2	2	8
	AB-40	2	2	2	2	8
	AB-46	2	2	2	2	8
	AB-49	2	2	2	2	8
	AB-50	2	3	2	1	8

Table 1, cont.: Allen Brook Watershed Problem Area Evaluation

Lower Priority	AB-02	2	1	2	2	7
	AB-16	2	1	2	2	7
	AB-35	1	2	2	2	7
	AB-36	2	1	2	2	7
	AB-41	1	2	2	2	7
	AB-47	2	1	2	2	7
	AB-48	2	2	2	1	7
	AB-04	2	1	2	1	6
	AB-14	1	2	2	1	6
	AB-21	1	2	2	1	6
	AB-38	1	2	2	1	6
	AB-42	1	2	1	2	6
	AB-52	2	1	2	1	6
	AB-03	1	1	2	1	5
	AB-10	1	2	1	1	5
	AB-33	1	1	2	1	5

1 Only sites that scored priority 5 or above are listed. Sites are listed in order of priority, with the highest priority first.

Table 2: Muddy Brook Watershed Problem Area Evaluation

		Relative Impact	Frequency	Current Conditions	Urgency	Priority ¹
High Priority	MB-15	3	2	3	3	11
	MB-26	3	2	2	3	10
	MB-18	2	2	2	3	9
	MB-02	2	2	2	2	8
	MB-08	2	2	2	2	8
	MB-11	2	2	2	2	8
	MB-23	2	2	2	2	8
Lower Priority	MB-10	2	2	1	2	7
	MB-12	1	2	2	2	7
	MB-13	2	2	1	2	7
	MB-14	2	2	2	1	7
	MB-19	1	2	2	2	7
	MB-27	2	1	2	2	7
	MB-07	1	1	2	2	6
	MB-09	1	1	2	2	6
	MB-20	2	2	1	1	6
	MB-03	2	1	1	1	5
MB-25	1	1	2	1	5	

1 Only sites that scored priority 5 or above are listed. Sites are listed in order of priority, with the highest priority first.

Table 3: Sucker Brook Watershed Problem Area Evaluation

		Relative Impact	Frequency	Current Conditions	Urgency	Priority
High Priority	SB-09	3	2	2	2	9
	SB-13	1	2	3	3	9
	SB-12	2	2	2	2	8
	SB-01	2	2	2	1	7

Table 4: Winooski Direct Drainage Problem Area Evaluation

		Relative Impact	Frequency	Current Conditions	Urgency	Priority ¹
High Priority	WR-01	3	3	3	3	12
	WR-04	3	3	3	2	11
	WR-03	3	2	2	2	9
	WR-05	2	2	2	2	8
	WR-08	2	2	2	2	8
	WR-06	2	2	2	1	7
	WR-02	1	2	1	1	5

¹ Only sites that scored priority 5 or above are listed. Sites are listed in order of priority, with the highest priority first.

5. EVALUATING POSSIBLE FUTURE CONDITIONS

The data collection and field observation approach described above is well-suited for identifying existing stormwater problem areas, but is not designed to anticipate impacts from future projected land uses within the Town. The ability to proactively identify areas of the landscape that may be particularly susceptible to changes in hydrology resulting from land use changes will enable the Town to develop strategies to avoid such impacts as opposed to being relegated to remediating problems only after they occur.

This sort of “future condition” analysis was completed for Allen Brook as part of the process used to develop the stormwater TMDL and draft Flow Restoration Plan (see Sections 3.1.1.2 and 3.1.1.6). To date, however, a similar evaluation has not been made in other areas of Williston. Given development patterns in Williston and the lower sensitivity of larger rivers (such as the Winooski River) to modest increases in stormwater flows, this effort was focused on the Muddy Brook/Sucker Brook watershed. A GIS-based approach was employed to estimate how stormwater flows may increase as a result of projected future land use and build-out changes within the watershed.

Before considering a build-out scenario, it is important to understand baseline environmental and development conditions and how those conditions relate to the mapped or total impervious area (TIA) and effective impervious area (EIA), in order to predict changes in stormwater runoff in the target watersheds. The distinction between TIA and EIA is that EIA recognizes that some impervious areas are completely surrounded by pervious areas and therefore have less of an impact on aquatic ecosystems. EIA is the impervious cover that provides stormwater flows fairly directly and quickly to streams.

As part of the current condition GIS-based design flow analysis, sub-watersheds were classified based on the type and amount of stormwater management infrastructure to estimate EIA. The Consultant Team combined parcel-specific information – linking spatial GIS datasets such as parcel boundaries, land cover, and soil properties with database information such as property use – with the estimated EIA to predict current flows.

The Consultant Team then incorporated results from the current condition analysis and the build-out analysis previously completed by the Chittenden County Regional Planning Commission (CCRPC) to determine areas of current and future development and the underlying soil, slope, and land cover conditions. This analysis was further modified based on the number of new buildings from the build-out analysis, and associated assumptions regarding residential building footprint size, land cover, and impervious surface. Estimated stormwater runoff volumes were determined based on the soil properties of the parcel, and the amount of impervious surface and land cover as a result of current and future development. The results were then used to rank estimated increases in stormwater flows at build-out in different subwatersheds within the Muddy Brook/Sucker Brook watershed (see Appendix C, Figure 1). The approach is further detailed in Appendix D.

For future planning and stormwater management purposes, the Town may wish to consider low-impact design requirements, zoning conditions, and other measures to further manage stormwater runoff – particularly on parcels that do not reach the regulatory threshold for state stormwater jurisdiction. Low impact design could help to minimize stormwater impacts from new projects that would not trigger state regulatory requirements in areas expecting to see the most growth. Currently, Williston’s *Unified Development Bylaw* requires only that developers disturbing more than ¼-acre retain runoff on-site to be “infiltrated and/or released at a rate not exceeding the pre-development rate of release.” More could be done to explicitly incentivize or require developers to employ low-impact design measures.

6. NEXT STEPS

This document is Phase 1 and represents an extensive effort to identify and evaluate potential stormwater problem areas throughout the Town of Williston, and has resulted in the identification of approximately 30 potential stormwater improvement projects that the Town could pursue (see Tables 1-3, above). These summary tables include a number of high priority projects that might be readily addressed by the Town (e.g., classification level “3” projects receiving a minimum “priority score” of 8). Additional information on two key factors that will influence follow-up work related to this plan are described below.

6.1. FY13 Ecosystem Restoration Program Grant

The Town of Williston has applied for and received a grant from VT DEC to complete Phase 2 of the *Williston Town-Wide Stormwater Master Plan* project. This work will more clearly organize and prioritize the information collected as part of developing the Problem Area Data Sheets, described in Section 3. Specifically, Phase 2 entails completing an engineering analysis of each of the critical problem areas including a preliminary cost estimate, as identified in Section 4.2, in order to prioritize these stormwater management needs. Phase 2 will include the design and implementation of a solution to address one of the identified problem areas. Tasks to be completed in the next phase of this work include:

- Provide specific recommendations for improved stormwater management based on existing conditions in the town;
- Prepare conceptual solutions for problem areas;
- Develop a basic implementation plan, based on the findings of the project, that prioritizes the problem areas based on anticipated water quality benefits in a single, town-specific, resource to guide future stormwater management activities; and
- Design and implement a solution for one of the problem areas.

Work on Phase 2 is anticipated to begin in early 2013.

6.2. MS4 Permit

On December 5, 2012, Vermont's MS4 Permit was issued. This MS4 permit is the second MS4 General Permit issued by the State of Vermont. The first MS4 permit was issued in 2003 and amended in 2004. The most significant change in the 2012 MS4 permit is the requirement for municipalities to develop Flow Restoration Plans (FRPs) to implement the TMDLs developed for stormwater-impaired watersheds. The FRPs must be developed for each impaired watershed within three years, and must include the following elements:

- Identification of the required controls,
- Design and construction schedule,
- A financial plan,
- A regulatory analysis,

- Identification of regulatory assistance, and
- Identification of any third party implementation.

The schedule needs to provide for implementation of the required BMPs as soon as possible, but within 20 years of the effective date of the permit.

The issuance of the MS4 permit has direct implications for stormwater management in Williston, in particular with regard to the implementation of the Allen Brook FRP. As was noted in Section 3.1.1.2 above, a draft FRP has been prepared that addresses the elements identified here.

6.3. Lake Iroquois Evaluation

The Town's 2011 Comprehensive Plan, Chapter 11.4, contains two key goals and actions concerning Lake Iroquois:

- Support the efforts of the Lake Iroquois Association, through direct funding and in-kind contributions, to help them achieve their mission to restore the lake.
- Develop a Lake Iroquois District Overlay to protect water quality, particularly with regard to the potential conversion of seasonal camps to year-round use where the existing camps do not conform to Williston's current watershed protection buffer regulations.

Recent (2010) work by volunteers of the Lake Iroquois Association and staff of the State Department of Environmental Conservation identified ten areas along the lakeshore as potential input points for nutrients and stormwater. A more detailed assessment of these ten areas will be completed during Phase 2 of the *Williston Town-Wide Stormwater Master Plan* project.

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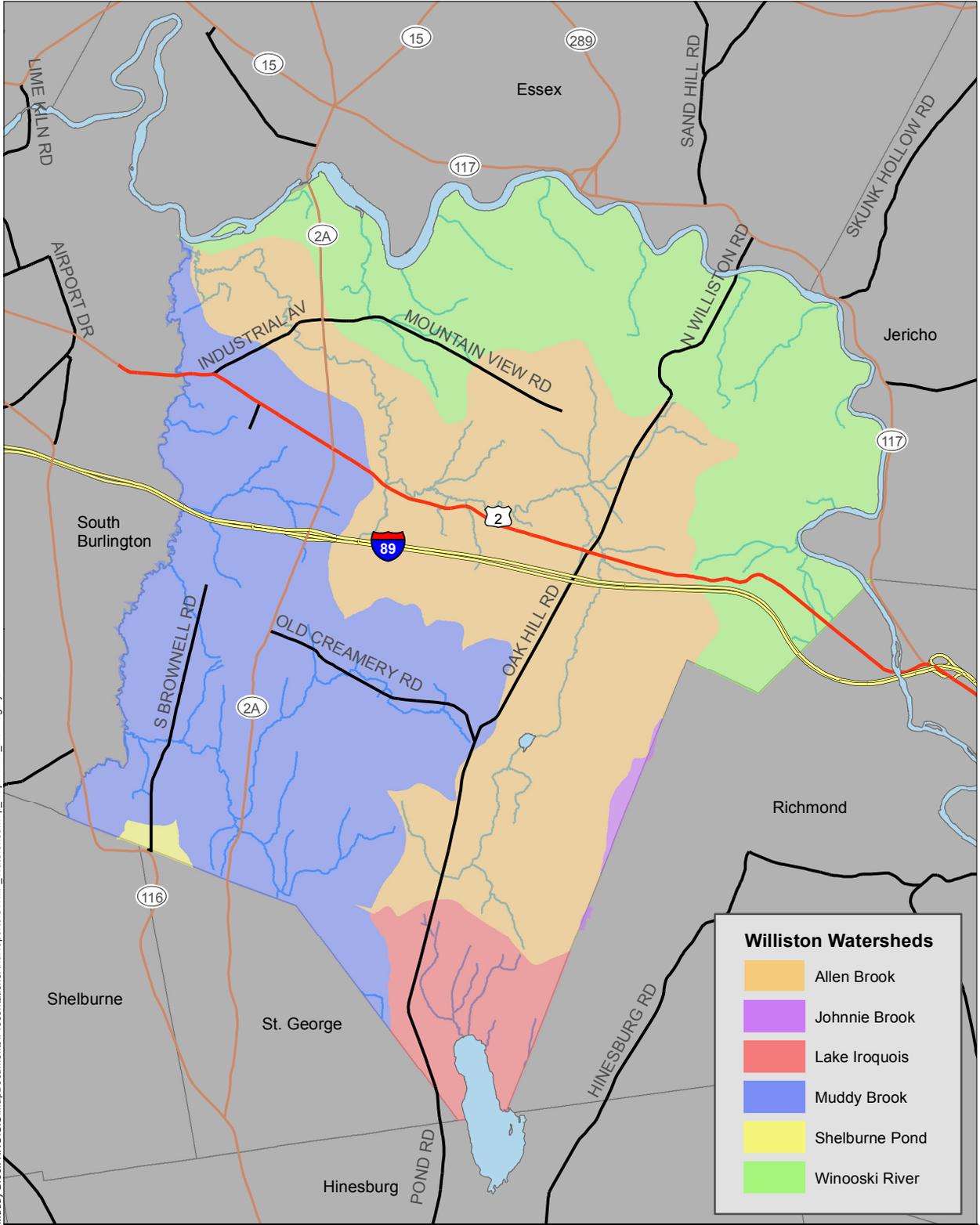
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APPENDICES

APPENDIX A: MAPS



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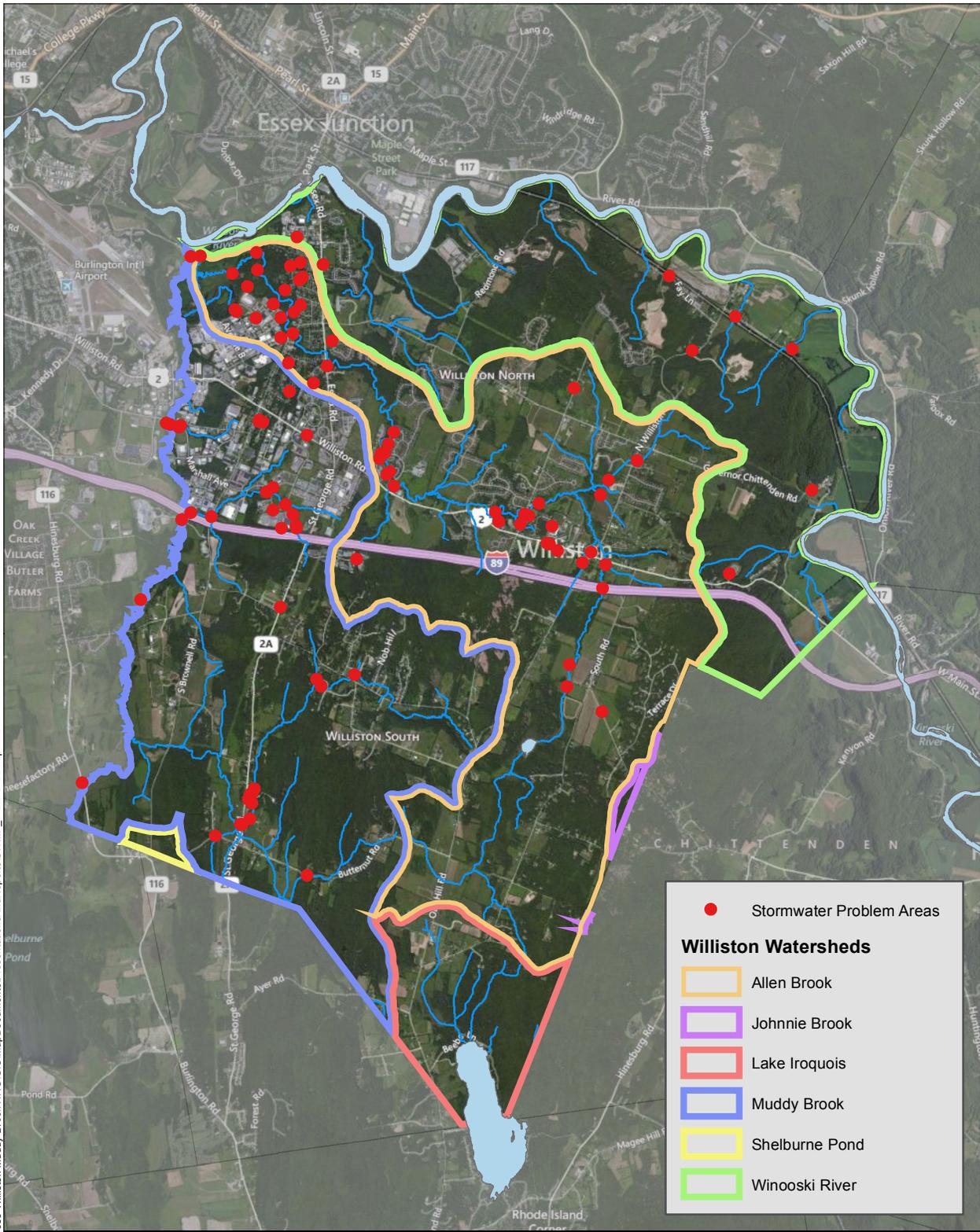
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Miles

Sources: Watershed Boundaries: Town of Williston; Administrative Boundaries, Hydrography, Roads: VCGI.

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Map 1: Watershed Boundaries

Williston Town-Wide
Stormwater Master Plan



- Stormwater Problem Areas

Williston Watersheds

- Allen Brook
- Johnnie Brook
- Lake Iroquois
- Muddy Brook
- Shelburne Pond
- Winooski River



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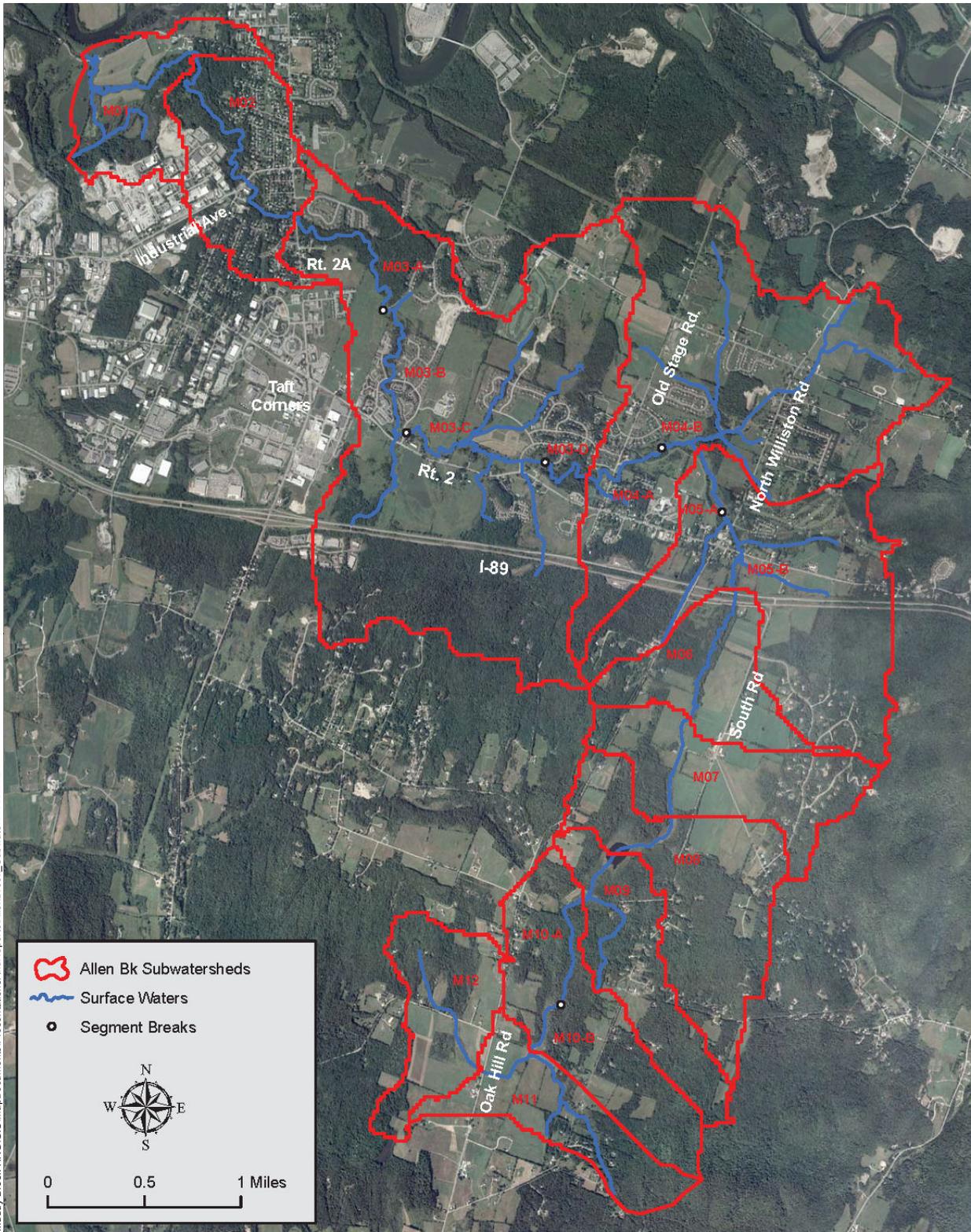
Sources: Stormwater Problem Areas: HRG;
Watershed Boundaries: Town of Williston;
Administrative Boundaries: VCGI.

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Map 2: Identified Stormwater Problem Areas in the Town of Williston

Williston Town-Wide
Stormwater Master Plan

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 Allen Bk Subwatersheds
 Surface Waters
 Segment Breaks

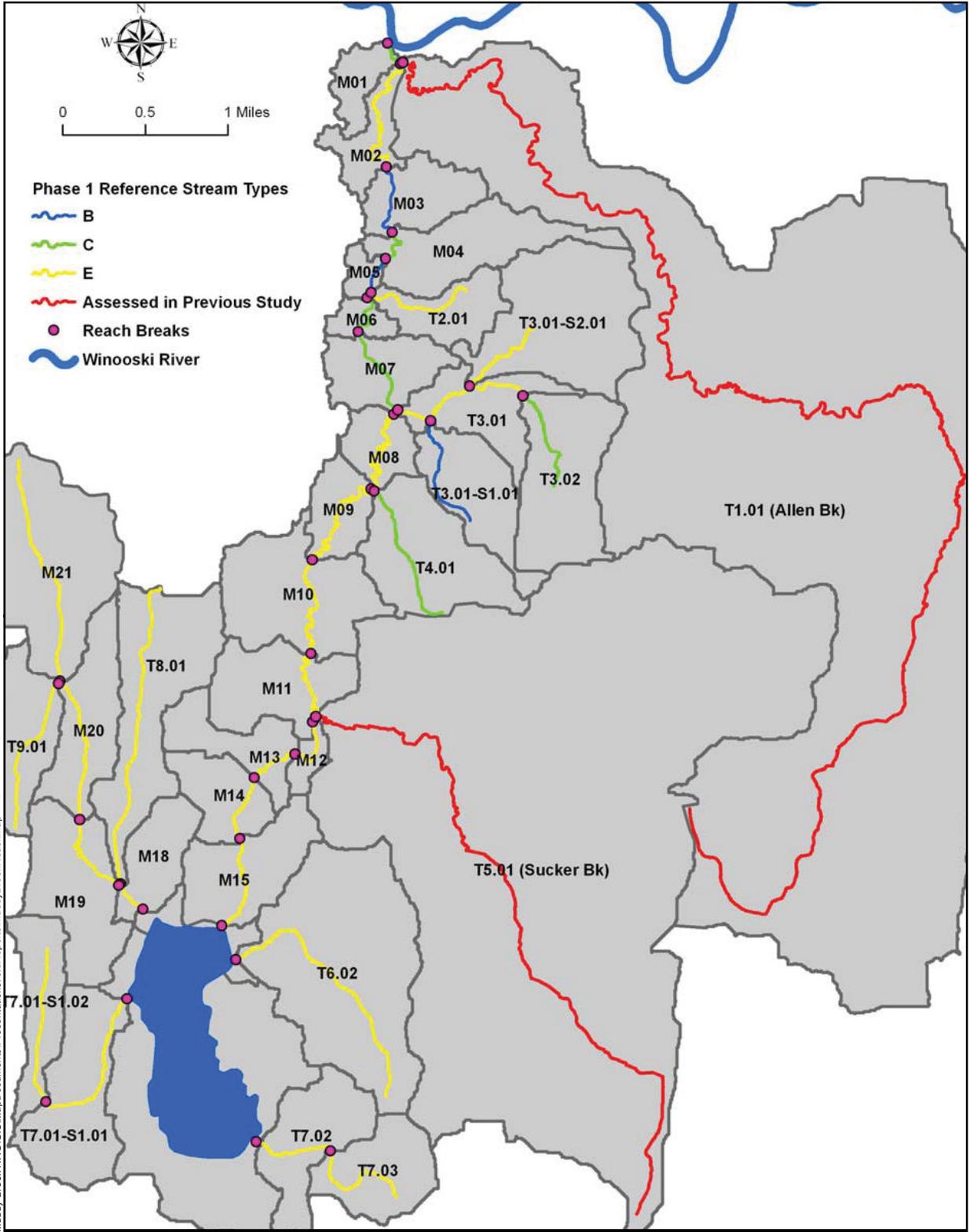


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Sources: Watershed Boundaries: Fitzgerald Environmental Associates.

Map 3: Allen Brook Subwatersheds and Stream Reaches



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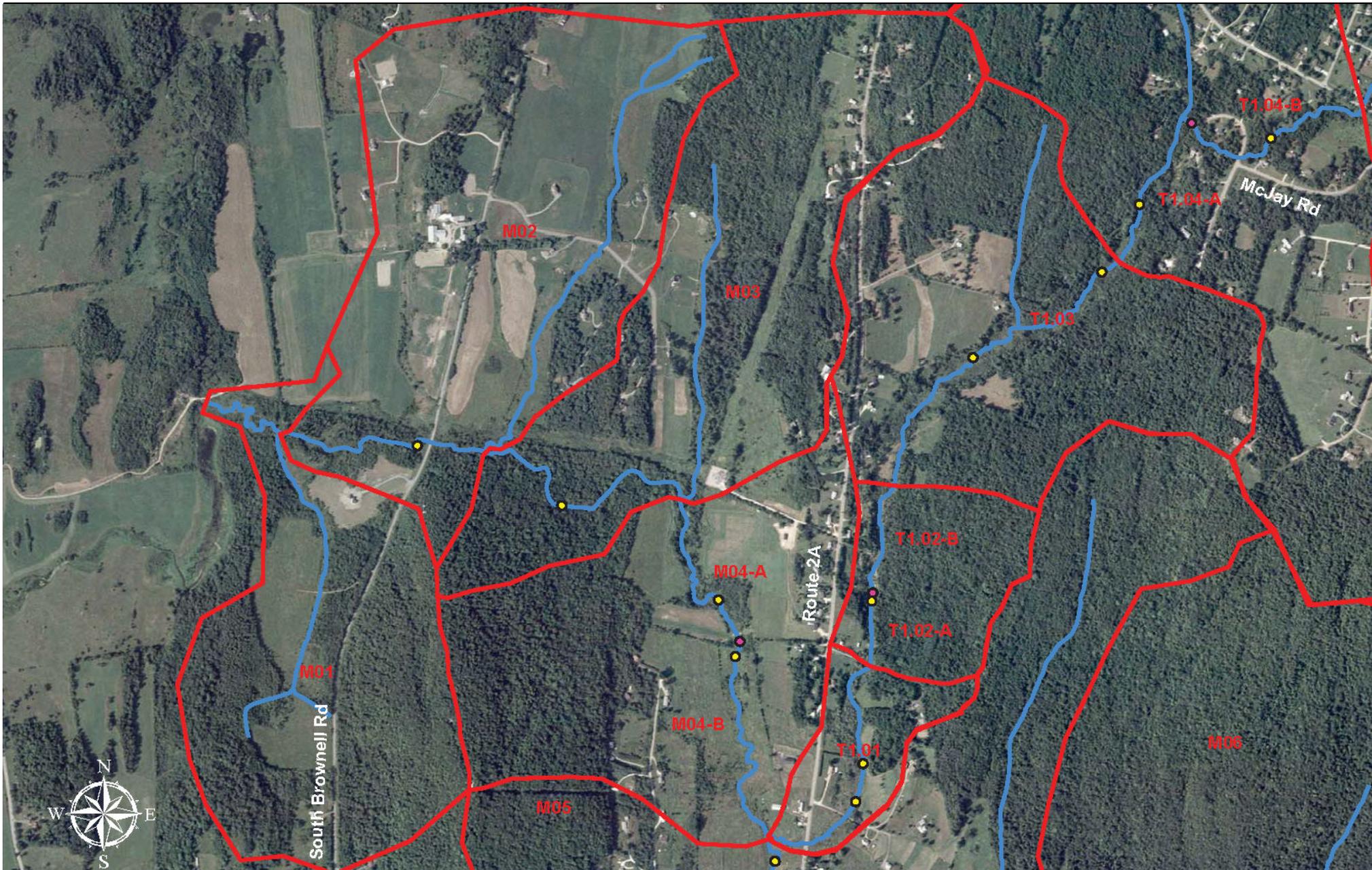


Sources: Watershed Boundaries:
Fitzgerald Environmental Associates

 **STONE ENVIRONMENTAL INC**

Map 4: Muddy Brook
Subwatersheds and Stream Reaches

Williston Town-Wide
Stormwater Master Plan



- Sucker Brook Cross Sections
- Sucker Brook Segment Breaks
- ~ Sucker Brook Surface Waters
- ⬮ Sucker Brook Subwatershed Boundaries

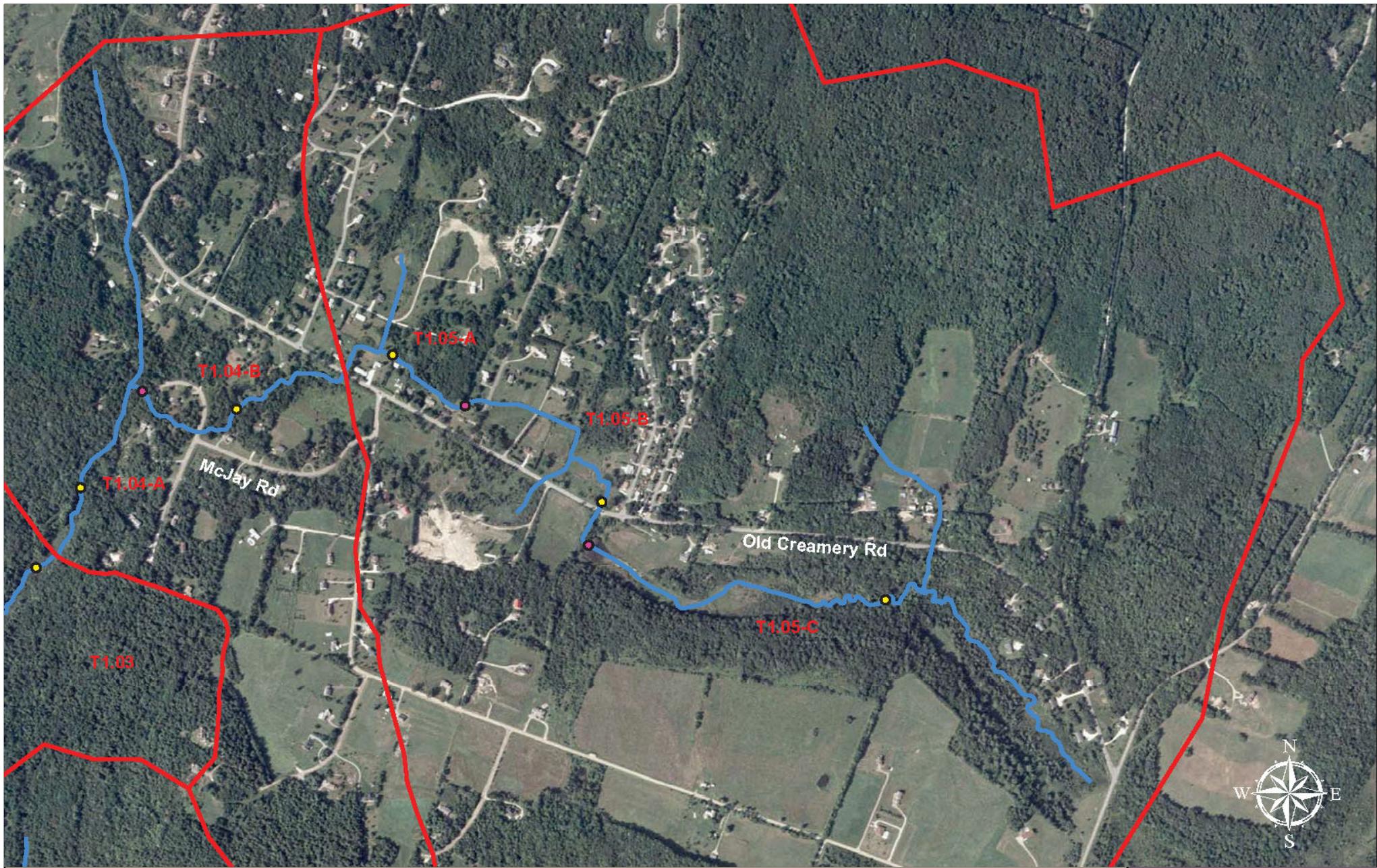
STONE ENVIRONMENTAL INC.

Sources: Sucker Brook Subwatershed data: Fitzgerald Environmental Associates



Map 5: Sucker Brook Subwatersheds and Stream Reaches

Williston Town-Wide Stormwater Master Plan



- Sucker Brook Cross Sections
- Sucker Brook Segment Breaks
- ~ Sucker Brook Surface Waters
- ⬮ Sucker Brook Subwatershed Boundaries

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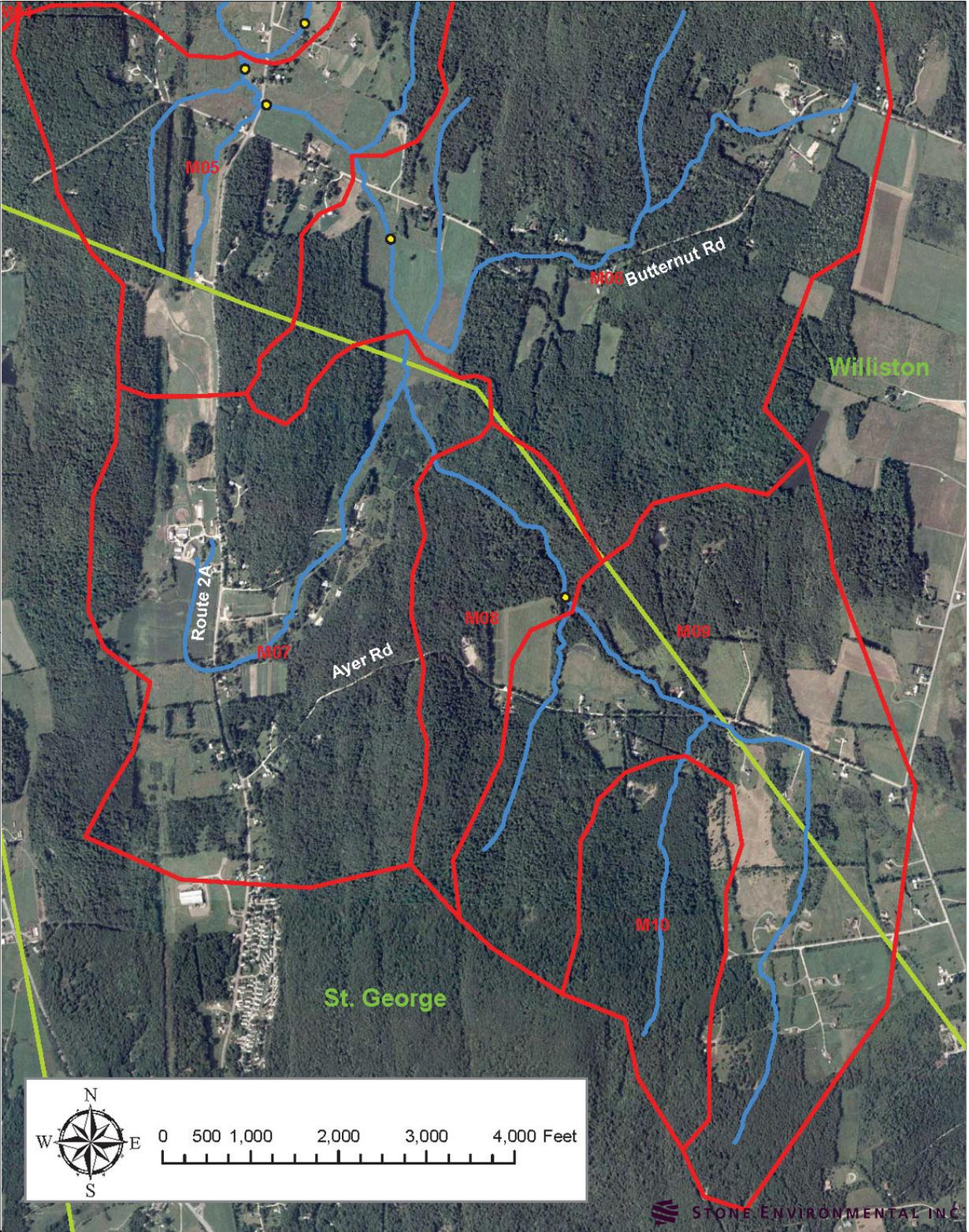
Sources: Watershed Boundaries:
Fitzgerald Environmental Associates



Map 6: Sucker Brook Subwatersheds and Stream Reaches

Williston Town-Wide Stormwater Master Plan

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- Sucker Brook Cross Sections
- Sucker Brook Segment Breaks
- Sucker Brook Surface Waters
- ⬮ Sucker Brook Subwatershed Boundaries

Sources: Watershed Boundaries: Fitzgerald Environmental Associates

Map 7: Sucker Brook
Subwatersheds and
Stream Reaches

Williston Town-Wide
Stormwater Master Plan

APPENDIX B: SWMP CITIZEN QUESTIONNAIRE

Citizen Questionnaire

Town of Williston Town-Wide Stormwater Master Plan

The Town is seeking information from residents about stormwater problems on your property and other locations throughout town. This information will be used to develop a stormwater study that is currently being completed to help shape the Town's strategy for how stormwater can be managed more effectively throughout Williston. Please take a few minutes to check the appropriate answer and write comments where needed.

Thank you in advance for your input.

PLEASE COMPLETE THE FOLLOWING QUESTIONNAIRE AND RETURN BY OCTOBER 31, 2011 TO:

Adam R. Zahniser, PE
Herbert, Rowland & Grubic, Inc.
3755 East State Street
Hermitage, PA 16148

Or, send by email to: azahniser@hrg-inc.com

PERSON COMPLETING QUESTIONNAIRE (OPTIONAL; May be used if needed to contact you for additional information)

Name:	
Owner Address:	
Property Address: (If different from above)	
Phone:	
e-mail:	
How long have you owned or lived at this location?	<input type="checkbox"/> Less than 1 year <input type="checkbox"/> 1 - 5 years <input type="checkbox"/> 5 - 15 years <input type="checkbox"/> More than 15 years

Local Drainage Problems

1. Have you experienced flooding or other drainage problems caused by stormwater?	<input type="checkbox"/> Yes <input type="checkbox"/> No
1.1. If yes, describe the location:	
1.2. If yes, how frequently does the stormwater problem occur at this location?	<input type="checkbox"/> Less than once per year <input type="checkbox"/> Once per year <input type="checkbox"/> More than once per year
1.3. If yes, does it:	
1.3.1. Flood yards or other open space with little or no damage?	<input type="checkbox"/> Yes <input type="checkbox"/> No <input type="checkbox"/> N/A
1.3.2. Damage public infrastructure such as roads or utility lines?	<input type="checkbox"/> Yes <input type="checkbox"/> No <input type="checkbox"/> N/A
1.3.3. Damage the basement of your primary structure?	<input type="checkbox"/> Yes <input type="checkbox"/> No <input type="checkbox"/> N/A
1.3.4. Damage other private property such as landscaping, or accessory structures?	<input type="checkbox"/> Yes <input type="checkbox"/> No <input type="checkbox"/> N/A
1.4. If water enters your home (primary structure) does it enter through the (please check all that apply):	
<input type="checkbox"/> Floor <input type="checkbox"/> Walls <input type="checkbox"/> Drains <input type="checkbox"/> Bathtub <input type="checkbox"/> Toilet <input type="checkbox"/> Windows <input type="checkbox"/> Window-Wells <input type="checkbox"/> N/A	
2. Where does the excess stormwater that causes this problem come from?	
Street or Road (name)	
Creek or Stream (name)	
Adjacent Property (describe)	
Other Sources (list)	
3. Are you aware of any other problems with the storm drainage system (i.e., ditches, pipes, drains, or streams)?	<input type="checkbox"/> Yes <input type="checkbox"/> No
3.1 If yes, check all situations that apply.	
<input type="checkbox"/> Corroded pipes <input type="checkbox"/> Pipe blockage <input type="checkbox"/> Drains in need of repair <input type="checkbox"/> Stream or ditch blockage <input type="checkbox"/> Other _____	

Stream Flooding Problems	
4. Do you know of flooding that occurs as a result of streams / rivers overflowing their banks?	<input type="checkbox"/> Yes <input type="checkbox"/> No
4.1. If yes, describe the location:	
4.2. If yes, how frequently does flooding occur at this location?	<input type="checkbox"/> Less than once per year <input type="checkbox"/> Once per year <input type="checkbox"/> More than once per year
4.3. If yes, does it:	
4.3.1. Flood yards or other open space with little or no damage?	<input type="checkbox"/> Yes <input type="checkbox"/> No <input type="checkbox"/> N/A
4.3.2. Damage private property such as landscaping, accessory structures, or homes?	<input type="checkbox"/> Yes <input type="checkbox"/> No <input type="checkbox"/> N/A
4.3.3. Damage public infrastructure such as roads or utility lines?	<input type="checkbox"/> Yes <input type="checkbox"/> No <input type="checkbox"/> N/A

Erosion Problems	
5. Do you know of any soil erosion problems from a storm drainage system (i.e. ditches, pipes, drains, or streams)?	<input type="checkbox"/> Yes <input type="checkbox"/> No
5.1. If yes, please indicate the location and severity of the problem(s):	
Location:	<input type="checkbox"/> Minor <input type="checkbox"/> Moderate <input type="checkbox"/> Severe
Location:	<input type="checkbox"/> Minor <input type="checkbox"/> Moderate <input type="checkbox"/> Severe
Location:	<input type="checkbox"/> Minor <input type="checkbox"/> Moderate <input type="checkbox"/> Severe

Water Quality Problems	
6. Are you aware any sources of pollution such as trash, sediment, fertilizer, or other chemicals entering a storm drainage system (i.e. ditches, pipes, drains, or streams)?	<input type="checkbox"/> Yes <input type="checkbox"/> No
6.1. If yes, please indicate the location and describe the nature of problem(s):	
Location:	Description:
Location:	Description:
Location:	Description:

Please describe in detail any flood-related or stormwater-related problems that have not already been described above:

Do you have any photographs, videotape, or other records of flooding problems that occurred on your property or in your neighborhood? If so, please send to the address listed above on this questionnaire. Any documentation or media that you provide will be used to help complete this study.

<input type="checkbox"/> No	<input type="checkbox"/> Video
<input type="checkbox"/> Written	<input type="checkbox"/> Other _____
<input type="checkbox"/> Photos	

APPENDIX C: PROBLEM AREA DATA SHEETS

C.1. Allen Brook

Problem Area Data Sheet

Problem Area ID: AB-01	Latitude: 44.472829 ° N	Longitude: 73.133329 ° W
Watershed: Allen Brook <hr/> Location: River Cove Rd, 1 mi. west of Route 2A <hr/> Problem Type: Local Drainage <hr/> Identification Source: Allen Brook SGA <hr/> Ownership: Public (Town)		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:
 Bridge appears to be in good condition. During flood stages bridge width may be an issue. Bridge is 73% of bankfull channel width. Significant sedimentation has “smothered” rock substrate in channel; source of sediment was not immediately apparent.

SWMP Field Assessment Photos

	
<p>Photo 1. Evidence of significant sedimentation</p>	<p>Photo 2. Downstream view of River Cove Road bridge during flood stage (4/28/2012)</p>

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
3	2	1	2	Yes	No	Yes	Yes

Problem Area Data Sheet

Problem Area ID: AB-02	Latitude: 44.470772 ° N	Longitude: 73.127990 ° W
Watershed: Allen Brook Location: Griswold Property b/w Industrial Park- Ave C and River Cove Rd Problem Type: Channel Erosion Identification Source: Allen Brook SGA Ownership: Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

Historical straightening of Allen Brook channel as part of the adjacent agricultural operation led to channel incision and increased bed and bank erosion that continues today. Numerous instances of eroding channels banks and tall adjacent terraces that are actively eroding into the channel. Several corrective measures have been implemented, but more work remains to be done.

Field Photos



Photo 1.

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	1	2	2	No	No	Yes	Yes

Problem Area Data Sheet

Problem Area ID: AB-03	Latitude: 44.473282 ° N	Longitude: 73.124069 ° W
Watershed: Allen Brook		
Location: River Cove Rd, 0.5 mi. west of Route 2A		
Problem Type: Channel Erosion		
Identification Source: Allen Brook SGA		
Ownership: Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:
 Concrete support blocks on North side of bridge have failed and collapsed into brook. Bridge does not appear to be causing a mid-channel deposition in brook as was noted in the SGA.

Field Photos



Photo 1. Support blocks in stream channel

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
1	1	2	1	No	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-04	Latitude: 44.471239 ° N	Longitude: 73.123822 ° W
Watershed: Allen Brook		
Location: North of industrial complex, at the west of the end of Sundown Dr.		
Problem Type: Channel Erosion		
Identification Source: Allen Brook SGA		
Ownership: Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

Aggradation has caused new (braided) channel to form; visible erosion on outer bank. Large riparian buffer zone, including some woody vegetation, present.

Field Photos



Photo 1. Visible erosion/undercutting of bank



Photo 2. Evidence of aggradation

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	1	2	1	No	No	Yes	Yes

Problem Area Data Sheet

Problem Area ID: AB-05	Latitude: 44.472150 ° N	Longitude: 73.116677 ° W
Watershed: Allen Brook		
Location: Palmer Court		
Problem Type: Local Drainage		
Identification Source: SWMP Field Assessment		
Ownership: Public (Town)/Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

Lawns on east-side of circle all slope towards road, and water reaching the road has no place to drain to. No catch basins or other stormwater collection devices (such as rain gardens) in area. Catch basins in road seem to have house drains flowing into them. Lawns drain to cul-de-sac with no stormwater facilities to collect runoff.

SWMP Field Assessment Photos



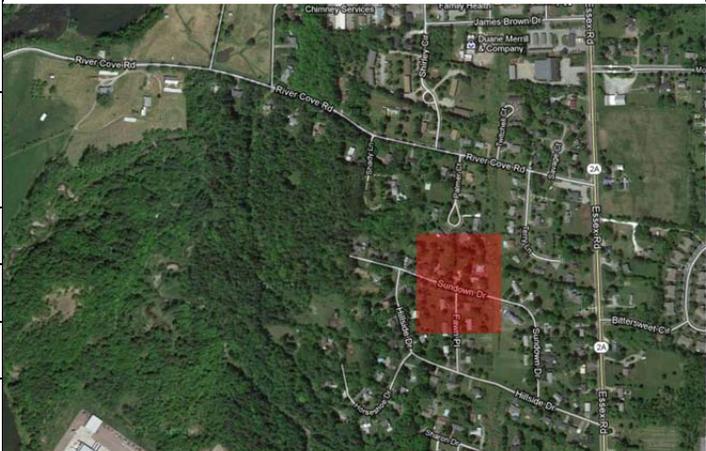
Photo 1. Stormwater runoff flowing into the cul-de-sac from surrounding residential properties.



Photo 2. Runoff ponding in the cul-de-sac and overflowing to the north along Palmer Court.

Relative Impact							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
1	3	2	2	Yes	Yes	Yes	Yes

Problem Area Data Sheet

Problem Area ID: AB-07	Latitude: 44.470441 ° N	Longitude: 73.116337 ° W
Watershed: Allen Brook		
Location: Fawn Place and Sundown Dr.		
Problem Type: Local Drainage		
Identification Source: Public Works Department		
Ownership: Private		

Date of Field Data Collection: 7/11/2012, 8/9/2012

Description of Observed Conditions:

Water is apparently coming from Sundown Dr; there are no catch basins or other drainage infrastructure on Fawn Pl. A curb cut was poorly repaired leading to water running on road to jump curb and flow through the lawns on Fawn Pl, into driveways and then being homes on Fawn Place. Residents indicated runoff eventually reaches the backyard to house on 264 Hillside Dr. Owners have installed a French drain to protect house and foundation from flooding. Owner at 55 Fawn Place is very concerned about flooding and damage to driveway.

Field Photos



Photo 1. Looking south along Fawn Place

Photo 2. Owner installed catch basin/leach field at 264 Hillside

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	2	2	Yes	Yes	Yes	Yes

Problem Area Data Sheet

Problem Area ID: AB-09	Latitude: 44.468854 ° N	Longitude: 73.119234 ° W
Watershed: Allen Brook <hr/> Location: Horseshoe Drive <hr/> Problem Type: Local Drainage <hr/> Identification Source: SWMP Field Assessment <hr/> Ownership: Public (Town)/Private		

Date of Field Data Collection: 04/28/2011

Description of Observed Conditions:

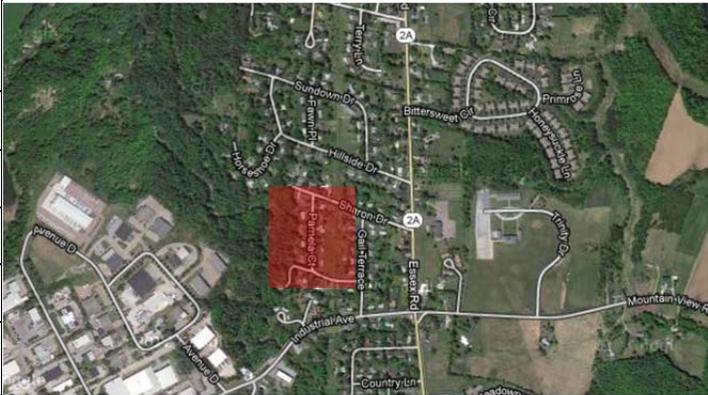
Catch basins at end of pavement are clogged with sediment, causing localized flooding. Catch basin on top of hill (in lawn) appears to be about 80% full of debris. French drain between houses on curve appears to be in good working order, channeling water from Sharon Dr. neighborhood. Discharges to an incline in woods that leads to Allen Brook, showing signs of gullyng.

SWMP Field Assessment Photos

Photo 1. Catch basin clogged with sediment.	Photo 2. Erosion of the gravel cul-de-sac.

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	3	2	Yes	Yes	Yes	Yes

Problem Area Data Sheet

Problem Area ID: AB-10	Latitude: 44.467116 ° N	Longitude: 73.116643 ° W
Watershed: Allen Brook		
Location: Pamela Court		
Problem Type: Local Drainage		
Identification Source: Resident report		
Ownership: Public/Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

Stormwater from Pamela Ct, Lea Dr, and N. Gail Terr all flows to culvert at intersection of Pamela and Lea (photo 1). Culvert is 18" corrugated metal pipe. At outfall, some sediment is building up; may contribute to localized road flooding reported by residents. Gail Terr catch basins are clear or debris. At 92 Gail, basin in yard is full of debris. No catch basins present on Pamela Ct, ditch drainage through culverts under driveways - most are 12" pipes.

Field Photos

	
Photo 1. Culvert at intersection of Gail/Pamela	Photo 2. Culvert outfall

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
1	2	1	1	Yes	Yes	Yes	No

Problem Area Data Sheet

Problem Area ID: AB-11	Latitude: 44.466269 ° N	Longitude: 73.117744 ° W
Watershed: Allen Brook		
Location: Forested area west of Horseshoe Dr		
Problem Type: Channel Erosion		
Identification Source: SWMP Questionnaire		
Ownership: Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

Channel is main run-off path for nearby neighborhood. Where the unnamed tributary enters the woods, severe erosion and overgrowth around channel is present. Incline in woods is steep and mass failures have occurred all the way to Allen Brook.

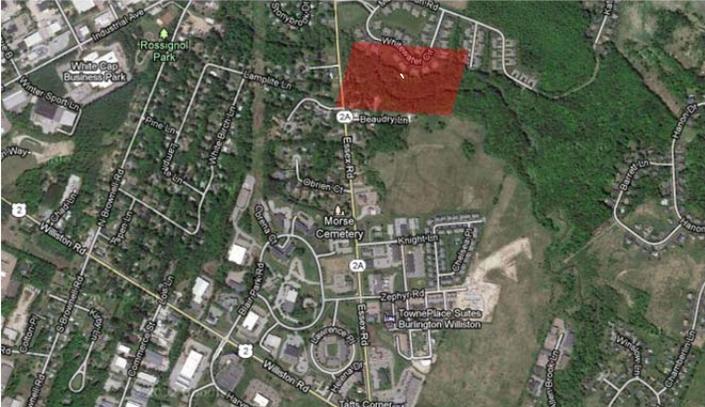
Field Photos



Photo 3. Overgrowth and undercutting **Photo 4.** Mass failure on approach to Allen Brook

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
3	2	2	2	No	Yes	No	Yes

Problem Area Data Sheet

Problem Area ID: AB-13	Latitude: 44.459864 ° N	Longitude: 73.112169 ° W
Watershed: Allen Brook		
Location: Route 2A and Hideaway Ln		
Problem Type: Overland Erosion		
Identification Source: SWMP Field Assessment		
Ownership: Public (VTrans)		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

Rip rap has been placed in the ditch/gully alongside road. Small headcut (<1') and channeling in lower half of rip rap. Some sediment deposition, presumably carried from road is present, where the rip rap ends (Photo 1).

Field Photos



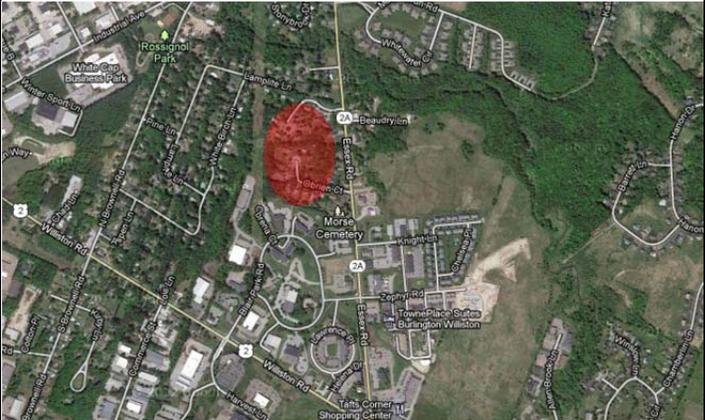


Photo 1. Looking down riprap

Photo 2. Looking up riprap, channeling

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	3	2	2	Yes	Yes	Yes	No

Problem Area Data Sheet

Problem Area ID: AB-14	Latitude: 44.457800 ° N	Longitude: 73.114360 ° W
Watershed: Allen Brook		
Location: O'Brien Court		
Problem Type: Local Drainage		
Identification Source: Public Works Department		
Ownership: Public (Town)		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

Catch basins at curve in road appear to have high water flow and sediment deposits during rain events. Signs of sediment and water flow down street towards the two catch basins. Catch basin grate is partially obstructed by accumulated sediment and debris.

Field Photos



Photo 1. **Photo 2.** Debris accumulated at catch basin inlet

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
1	2	2	1	Yes	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-16	Latitude: 44.445600 ° N	Longitude: 73.100988 ° W
Watershed: Allen Brook Location: East end of Talcott Rd, north of Route. 2 Problem Type: Channel Erosion Identification Source: Allen Brook SGA Ownership: Public		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

Culvert is significantly less than bankfull width. Signs of channel scour just below culvert (Photo 2). Rock, sediment and debris are being deposited at outfall.

Field Photos



Photo 1. Debris deposits

Photo 2. Scour pool

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	1	2	2	No	No	Yes	Yes

Problem Area Data Sheet

Problem Area ID: AB-17	Latitude: 44.447016 ° N	Longitude: 73.102131 ° W
Watershed: Allen Brook		
Location: Between Tyler Way and Seth Circle		
Problem Type: Overland Erosion		
Identification Source: Allen Brook Watershed Improvement Plan		
Ownership: Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

Water from parking lot causing erosion; some localized erosion spots on hillside facing buildings (Photo 1). Work underway for drainage on edge of parking lot. Water marks are visible on edge of pavement and grass on approach to wet pond drainage area. Channeling is beginning to undermine rip rap leading to pond as well (Photo 2).

Field Photos



Photo 1. Water Damage



Photo 2. Channeling in rip rap

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	2	2	No	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-18	Latitude: 44.447337 ° N	Longitude: 73.101646 ° W
Watershed: Allen Brook <hr/> Location: Between Tyler Way and Seth Circle <hr/> Problem Type: Local Drainage <hr/> Identification Source: Allen Brook Watershed Improvement Plan <hr/> Ownership: Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

12" plastic pipe outfalls to two- 6" pipes that appear to help direct flow of water. Pipes appear to have been buried at one point, but erosion has caused them to be exposed. Scour pool and channelization has formed at outfall of two 6" pipes. Water leads to a wet bank buffer zone before flowing into Allen Brook.

Field Photos

	
Photo 1. "Intersection" of 12" and 6" pipes	Photo 2. Scour at outfall of 6" pipes

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	3	2	No	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-19	Latitude: 44.448913 ° N	Longitude: 73.103306 ° W
Watershed: Allen Brook		
Location: Talcott Rd. South of Allen Brook Lane		
Problem Type: Overland Erosion		
Identification Source: Allen Brook Watershed Improvement Plan		
Ownership: Private		

Date of Field Data Collection: 7/11/2012 Note: Could be corrected through permit enforcement.

Description of Observed Conditions:

12" plastic outfall with broken end. Bank around outfall has failed and about 4' of pipe is exposed. Outfall drop is 4-5' to ground causing erosion issues. Major gully erosion present as water approaches Allen Brook. Sediment build up in Allen Brook, grass and weeds are growing on top of sediment causing a false bank to form in brook.

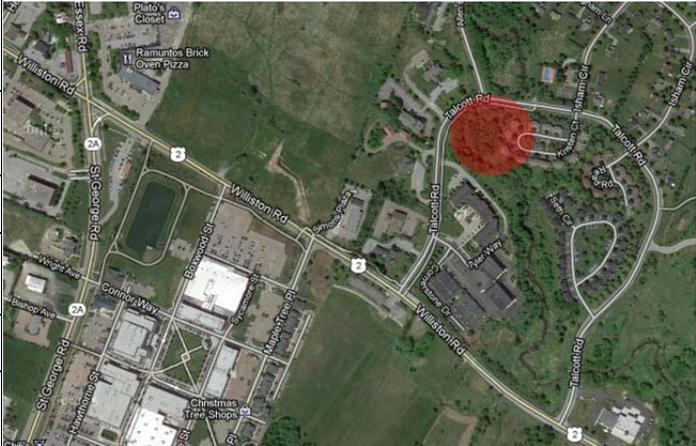
Field Photos

Photo 1. Failed bank and pipe outfall

Photo 2. Channel forming as water travels to Allen Brook

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
3	2	3	2	No	Yes	Yes	No

Problem Area Data Sheet

Problem Area ID: AB-20	Latitude: 44.449467 ° N	Longitude: 73.103128 ° W
Watershed: Allen Brook		
Location: Talcott Rd. South of Allen Brook Lane		
Problem Type: Overland Erosion		
Identification Source: Allen Brook Watershed Improvement Plan		
Ownership: Private		

Date of Field Data Collection: 7/11/2012 Note: Could be corrected through permit enforcement.

Description of Observed Conditions:
 12" plastic outfall; riprap has been placed below outfall - outfall drop is ~1'. Erosion is present in a few areas around riprap. Major sediment build up at intersection with Allen Brook. Large amounts of storm debris evident in brook.

Field Photos



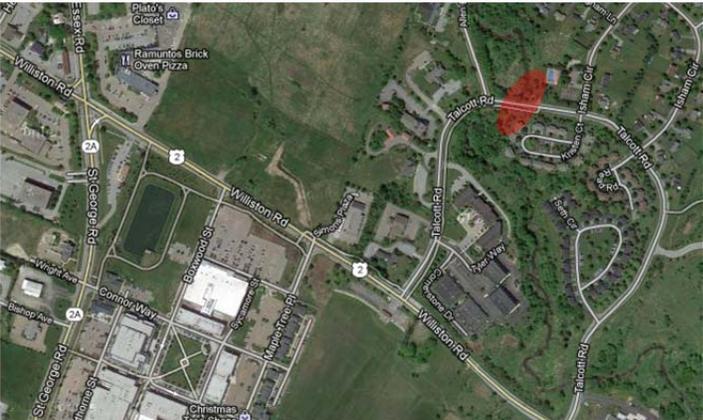
Photo 1. Erosion left of riprap



Photo 2. Sediment in Allen Brook

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	3	2	No	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-21	Latitude: 44.449588 ° N	Longitude: 73.102386 ° W
Watershed: Allen Brook Location: Talcott Rd. east of Allen Brook Lane Problem Type: Channel Erosion Identification Source: Allen Brook SGA Ownership: Public		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:
Sediment “delta” and scour pool forming at outfall of double-barrel culverts. Scour pool is developing at outfall. Culvert is less than bankfull width.

Field Photos	
	
Photo 1. Looking downstream from culvert outfall	Photo 2. Culvert outfall

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
1	2	2	1	No	No	Yes	Yes

Problem Area Data Sheet

Problem Area ID: AB-24	Latitude: 44.463698 ° N	Longitude: 73.117848 ° W
Watershed: Allen Brook Location: Industrial Ave. and NE of N. Brownell Rd. Problem Type: Local Drainage Identification Source: Allen Brook Watershed Improvement Plan Ownership: Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

Erosion is present between entire length of parking lot and Industrial Ave. Minor pavement trenching is present. Significant sediment accumulation on bridge that spans Allen Brook near north end of parking lot.

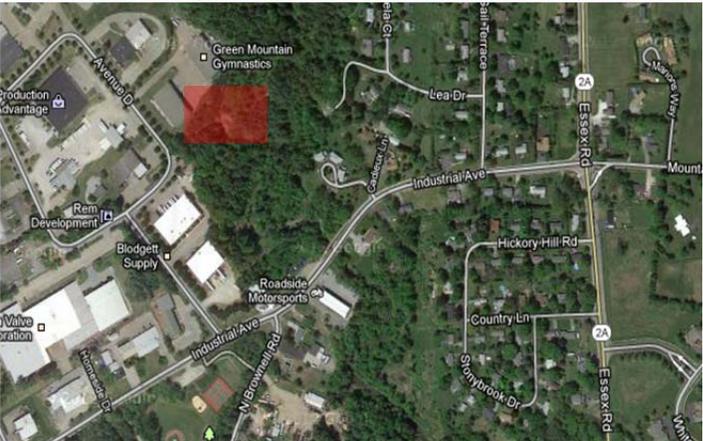
Field Photos

Photo 1. Erosion on Industrial Ave.

Photo 2. Sediment on bridge above Allen Brook

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	2	2	Yes	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-26	Latitude: 44.465566 ° N	Longitude: 73.119906 ° W
Watershed: Allen Brook		
Location: East of Avenue D		
Problem Type: Overland Erosion		
Identification Source: Allen Brook Watershed Improvement Plan		
Ownership: Private		

Date of Field Data Collection: 7/11/2012 Note: Could be corrected through permit enforcement.

Description of Observed Conditions:

Minor gullying at outfall; iron staining visible. Culvert is ~75% clogged with debris. No catch basins in parking lot. Outfall may be roof drains, gullying in woods may indicate parking lot run-off to woods.

Field Photos

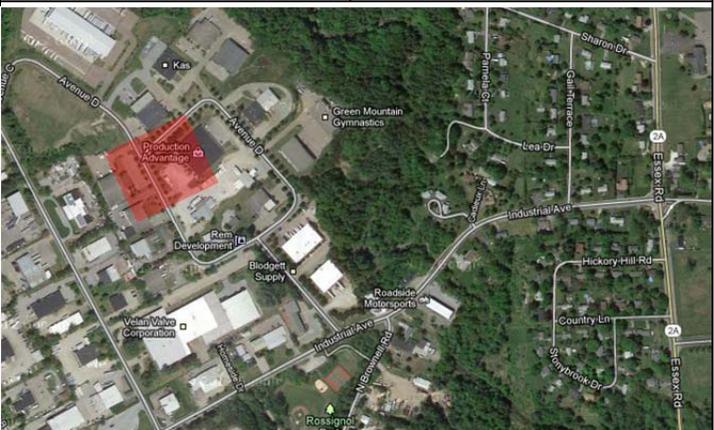


Photo 1. Blocked outfall, iron staining

Photo 2. Gully above outfall

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
1	2	3	2	No	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-29	Latitude: 44.465507 ° N	Longitude: 73.124015 ° W
Watershed: Allen Brook Location: Avenue D Problem Type: Overland Erosion Identification Source: Allen Brook Watershed Improvement Plan Ownership: Private		

Date of Field Data Collection: 7/11/2012 Note: Could be corrected through permit enforcement.

Description of Observed Conditions:

Erosion in drainage ditches on west side of Avenue D; excess flows have begun to undermine. Check dams have been installed to slow water rate as it approaches culvert. Gullying present on the east side of road at bend in Avenue D. Some evidence of hillside cutting and erosion on bank from parking lot run-off at Production Advantage.

Field Photos



Photo 1. Culvert, beginning to be undermined

Photo 2. Curve side trenching

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	2	2	Yes	Yes	Yes	No

Problem Area Data Sheet

Problem Area ID: AB-30	Latitude: 44.466510 ° N	Longitude: 73.127516 ° W
Watershed: Allen Brook		
Location: NW of end of Avenue C		
Problem Type: Landslide		
Identification Source: Public Works Department		
Ownership: Private		

Date of Field Data Collection: 7/11/2012

Note: Plans exist for resolution.

Description of Observed Conditions:

Major erosion and bank failure. Landslide extends several hundred feet down into old sand pit. Run-off from road could be contributing to erosion around outfall.

Field Photos



Photo 1. View from top of slide



Photo 2. Further down in slide

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	3	1	Yes	Yes	Yes	No

Problem Area Data Sheet

Problem Area ID: AB-32	Latitude: 44.457444 ° N	Longitude: 73.071198 ° W
Watershed: Allen Brook <hr/> Location: Old Stage Road south of Amber Ln <hr/> Problem Type: Local Drainage <hr/> Identification Source: SWMP Field Assessment <hr/> Ownership: Public		

Date of Field Data Collection: 6/24/11

Description of Observed Conditions:

Black 12" corrugated plastic culvert under Amber Ln. Sediment accumulating at outfall of culvert. Area is high point on Stage Rd where it appears that water may have no place to flow. Ditches are level, no gradient to them.

SWMP Field Assessment Photos



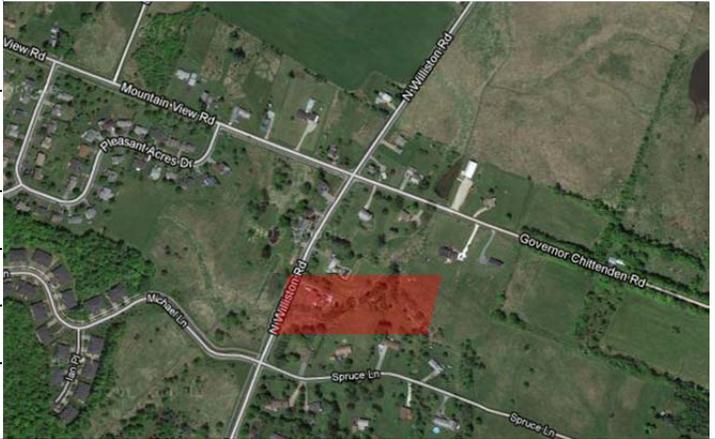
Photo 1. Water ponding along Old Stage Road at the intersection with Amber Lane



Photo 2. Water ponding along Amber Lane

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
1	3	2	2	Yes	Yes	Yes	No

Problem Area Data Sheet

Problem Area ID: AB-33	Latitude: 44.448864 ° N	Longitude: 73.060633 ° W
Watershed: Allen Brook		
Location: North Williston Rd, north of Spruce Ln		
Problem Type: Overland Erosion		
Identification Source: SWMP Questionnaire		
Ownership: Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:
 Ponding of water before entering marsh area. Pond is 8' by 12' about 8" deep. Signs of small dam/blockage at one point with rebar in stream; obstruction has been breached.

Field Photos

	
Photo 1. Ponding of water	Photo 2. Remnants of blockage

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
1	1	2	1	No	No	Yes	No

Problem Area Data Sheet

Problem Area ID: AB-34	Latitude: 44.446536 ° N	Longitude: 73.065333 ° W
Watershed: Allen Brook <hr/> Location: Between Ian Pl and North Williston Rd <hr/> Problem Type: Channel Erosion <hr/> Identification Source: Planning Office <hr/> Ownership: Private		

Date of Field Data Collection: 7/11/2012

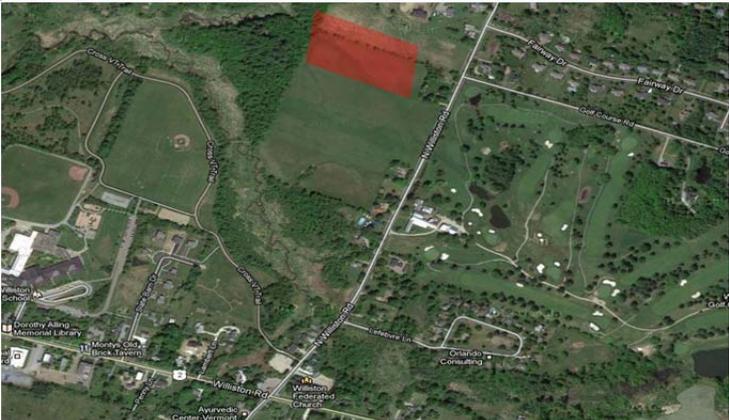
Description of Observed Conditions:
 Incised banks are constantly failing. Conditions worsen as stream progresses down channel. Large amount of development in the watershed.

Field Photos

	
Photo 1. Stream in wetland area	Photo 2. Erosion of stream banks.

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
3	1	2	2	No	Yes	No	Yes

Problem Area Data Sheet

Problem Area ID: AB-35	Latitude: 44.444751 ° N	Longitude: 73.066618 ° W
Watershed: Allen Brook <hr/> Location: North Williston Rd across from Golf Course Rd <hr/> Problem Type: Local Drainage <hr/> Identification Source: Planning Office <hr/> Ownership: Private		

Date of Field Data Collection: 7/11/2012

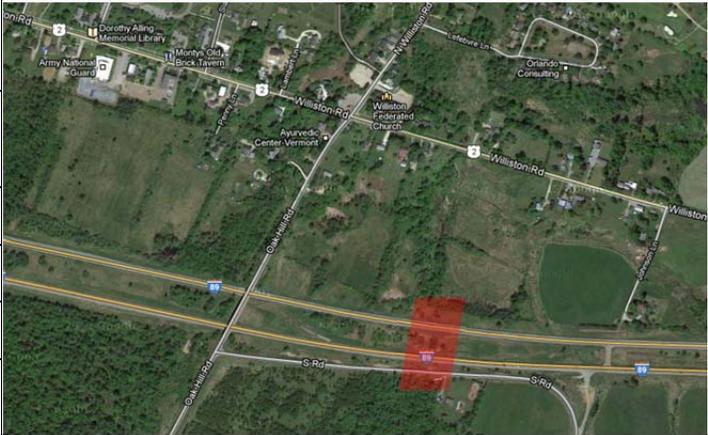
Description of Observed Conditions:

Loose gravel around underground pipe is eroding. Pipe end is filling in with material and channelization occurring immediately after outfall. Pipe appears to drain field and house area upstream. Problem caused by piping and burying the natural drainage which conveys runoff from the golf course.

Field Photos	
	
Photo 1.	Photo 2.

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
1	2	2	2	No	Yes	No	Yes

Problem Area Data Sheet

Problem Area ID: AB-36	Latitude: 44.433674 ° N	Longitude: 73.066224 ° W
Watershed: Allen Brook		
Location: I-89, 0.25 mi. west of Oak Hill Rd		
Problem Type: Channel Erosion		
Identification Source: Allen Brook SGA		
Ownership: Public (VTrans)		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:
 Upstream, brook makes an “S” as it approaches culverts. Bank scouring at the entrance to culvert. Between N/S lanes of 89 small scour pool is present. Scour pool at North bound outfall with 6” headcut from concrete box culvert.

Field Photos



Photo 1. **Photo 2.**

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	1	2	2	Yes	No	Yes	Yes

Problem Area Data Sheet

Problem Area ID: AB-38	Latitude: 44.436697 ° N	Longitude: 73.069556 ° W
Watershed: Allen Brook <hr/> Location: Private drive off of Oak Hill Rd, 0.35 mi. south of Rt. 2 <hr/> Problem Type: Local Drainage <hr/> Identification Source: Planning Office <hr/> Ownership: Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:
 Private drive washes into northeast side of stream. Road slopes towards the stream bringing road debris/gravel with it to stream. 3' corrugated culvert becoming overgrown with weeds; water flow is partially obstructed.

Field Photos

Photo 1. Overgrown culvert. Standing water	Photo 2. Overgrown weeds

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
1	2	2	1	No	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-39	Latitude: 44.437938 ° N	Longitude: 73.068192 ° W
Watershed: Allen Brook <hr/> Location: Northwest of the intersection of Route 2 and North Williston Rd <hr/> Problem Type: Local Drainage <hr/> Identification Source: Allen Brook Watershed Improvement Plan <hr/> Ownership: Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

Run-off from gravel parking lot flows to natural area/wetland between gas station parking lot and bike path parking lot. Sediment has built up along north side of parking lot. No catch basins in area.

Field Photos

Photo 1. Edge of parking lot

Photo 2. Ponding in front of natural areas

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	2	2	No	Yes	Yes	No

Problem Area Data Sheet

Problem Area ID: AB-40	Latitude: 44.438085 ° N	Longitude: 73.073711 ° W
Watershed: Allen Brook <hr/> Location: Route 2, 0.25 mi west of intersection with North Williston Rd <hr/> Problem Type: Local Drainage <hr/> Identification Source: Planning Office <hr/> Ownership: Public		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:
No catch basins/drainage ditch to provide water access to stormwater system. Signs of ponding and erosion on gravel drive and grass medians. Drainage ditch runs along south end of parking lot. Heavy truck and traffic use as town vehicles and National Guard use road.

Field Photos	
Photo 1. Edge of gravel lot to paved lot	Photo 2. Gravel lot

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	2	2	Yes	Yes	Yes	No

Problem Area Data Sheet

Problem Area ID: AB-41	Latitude: 44.438858 ° N	Longitude: 73.075100 ° W
Watershed: Allen Brook <hr/> Location: Route 2, approximately 0.35 mi west of the intersection with North Williston Rd <hr/> Problem Type: Local Drainage <hr/> Identification Source: Planning Office <hr/> Ownership: Private		

Date of Field Data Collection: 7/11/2012; 8/9/2012

Description of Observed Conditions:

There is a lawn swale that runs from under barn to edge of property and sidewalk. Some signs of standing water and localized erosion in private drive.

Field Photos



Photo 1.



Photo 2.

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
1	2	2	2	Yes	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-42	Latitude: 44.438945 ° N	Longitude: 73.075513 ° W
Watershed: Allen Brook		
Location: Route 2, approximately 0.35 mi west of the intersection with North Williston Rd		
Problem Type: Local Drainage		
Identification Source: Planning Office		
Ownership: Private		

Date of Field Data Collection: 7/11/2012; 8/3/2012

Description of Observed Conditions:
Stormwater pipe flows from French drain underground to catch basin. French drain is a 6" plastic pipe and stormwater pipe is 12". Signs of erosion and water bypass at French drain outlet causing water to flow into driveway and onto sidewalk and street.

Field Photos	
	
Photo 1.	Photo 2. Looking down to street

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
1	2	1	2	Yes	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-43	Latitude: 44.441017 ° N	Longitude: 73.074658 ° W
Watershed: Allen Brook <hr/> Location: Northeast corner of school complex on Central School Dr <hr/> Problem Type: Local Drainage <hr/> Identification Source: Allen Brook Watershed Improvement Plan <hr/> Ownership: Public		

Date of Field Data Collection: 7/11/2012 Note: Could be corrected through permit enforcement.

Description of Observed Conditions:

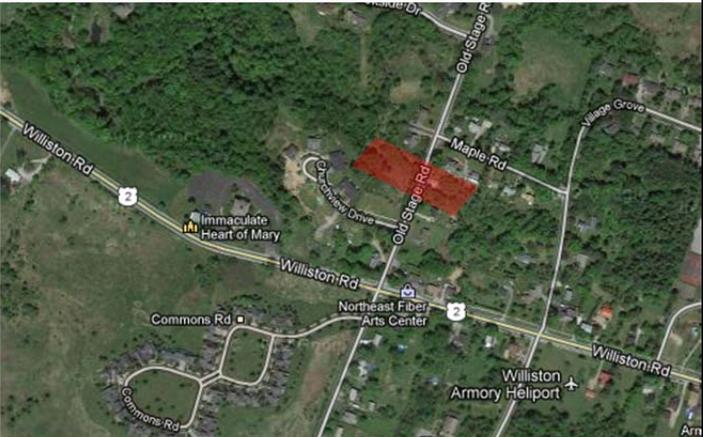
Swale is becoming incised about halfway through. Incision is approximately 1.5' deep. The 8" plastic pipe moving water from wet pond to swale is damaged and erosion is evident at outfall, as water drops 8" into swale.

Field Photos

Photo 1. Erosion in swale	Photo 2. Damaged outfall pipe

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	3	2	No	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-44	Latitude: 44.441194 ° N	Longitude: 73.079880 ° W
Watershed: Allen Brook		
Location: Old Stage Road just south of Maple Rd		
Problem Type: Local Drainage		
Identification Source: Public Works Department		
Ownership: Public		

Date of Field Data Collection: 7/11/2012

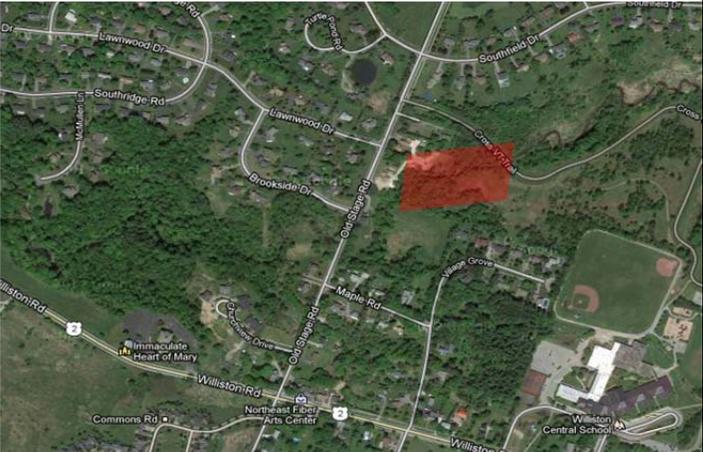
Description of Observed Conditions:
 3' corrugated metal culvert; bottom has deteriorated. Culvert is ~50% of bank width; scour is evident at outfall and channel is incising.

Field Photos

	
Photo 1. Inlet, grass on sides matted down	Photo 2. Deteriorated culvert

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	3	2	Yes	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-45	Latitude: 44.443656 ° N	Longitude: 73.076827 ° W
Watershed: Allen Brook		
Location: Old Stage Rd, directly across from Lawnwood Dr		
Problem Type: Channel Erosion		
Identification Source: Allen Brook SGA		
Ownership: Private		

Date of Field Data Collection: 8/3/2012

Description of Observed Conditions:
 Mass wasting and erosion on outer bank of stream; streambanks are undercut. Local, sandy soils may be particularly sensitive to unraveling.

Field Photos



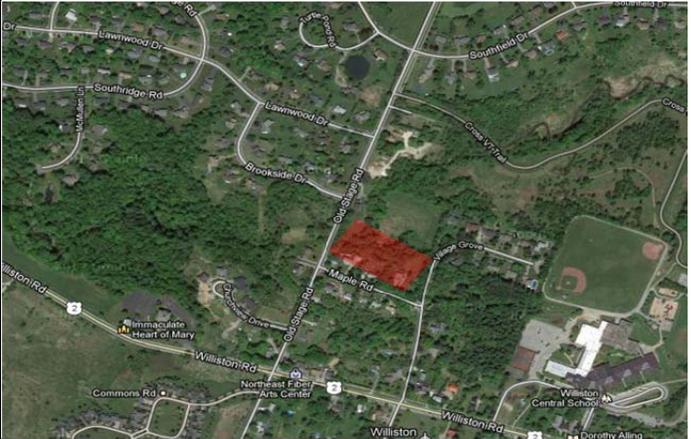
Photo 1.



Photo 2.

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
3	2	2	2	No	No	Yes	Yes

Problem Area Data Sheet

Problem Area ID: AB-46	Latitude: 44.442238 ° N	Longitude: 73.078499 ° W
Watershed: Allen Brook		
Location: Old Stage Rd, just north of Maple Rd		
Problem Type: Channel Erosion		
Identification Source: Allen Brook SGA		
Ownership: Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:
Bank erosion and undercutting. Old metal pipe of unknown discharging into brook; iron staining present.

Field Photos	
	
Photo 1. Bank cutting	Photo 2. Old metal pipe outlet

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	2	2	No	No	No	Yes

Problem Area Data Sheet

Problem Area ID: AB-47	Latitude: 44.442399 ° N	Longitude: 73.079245 ° W
Watershed: Allen Brook		
Location: Old Stage Rd, near Lawnwood Dr		
Problem Type: Channel Erosion		
Identification Source: Allen Brook SGA		
Ownership: Public		

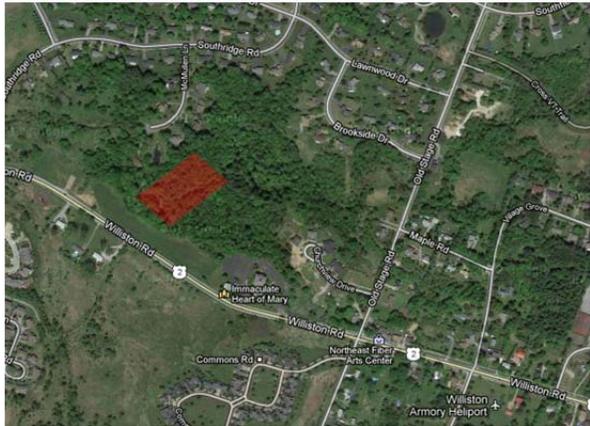
Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:
Large scour pool at outlet; standing water approximately 1.5' deep in pool. New bank work and rip rap placed at outlet. Possible start of undermining on upstream inlet.

Field Photos	
	
Photo 1. Large scour pool	Photo 2. Inlet, with possible undermining

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	1	2	2	Yes	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-48	Latitude: 44.442636 ° N	Longitude: 73.084041 ° W
Watershed: Allen Brook <hr/> Location: North of Route 2, near McMullen Ct <hr/> Problem Type: Channel Erosion <hr/> Identification Source: Allen Brook SGA <hr/> Ownership: Private <hr/>		

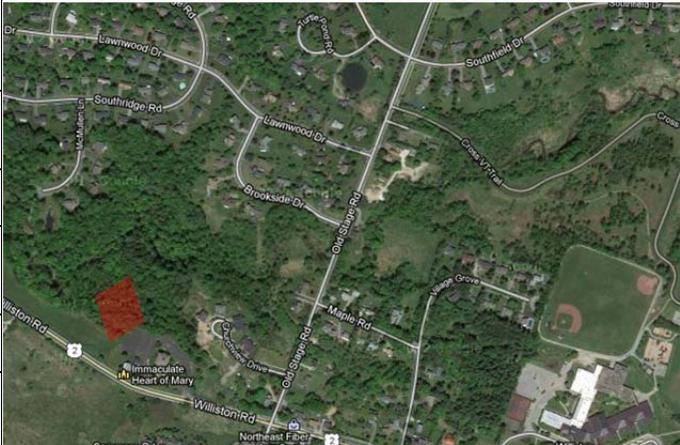
Date of Field Data Collection: 8/3/2012

Description of Observed Conditions:
Areas of streambank erosion and small undercuts and bank collapses were observed. One area where water has jumped bank was also noted.

Field Photos	
	
Photo 1. Streambank has been undermined	Photo 2. Localized bank collapses

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	2	1	No	No	No	Yes

Problem Area Data Sheet

Problem Area ID: AB-49	Latitude: 44.441489 ° N	Longitude: 73.083500 ° W
Watershed: Allen Brook Location: Route 2, west of Old Stage Rd Problem Type: Overland Erosion Identification Source: Allen Brook Watershed Improvement Plan Ownership: Private		

Date of Field Data Collection: 7/11/2012

Description of Observed Conditions:

Gully has formed in northwest corner of parking lot, where land slopes down to the woods line. Gully is not deep, but has potential to increase in size. Gully stops about 30 ft down into woods.

Field Photos



Photo 1. Looking up gully

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	2	2	2	No	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-50	Latitude: 44.424551 ° N	Longitude: 73.071560° W
Watershed: <u>Allen Brook</u> Location: <u>West of the intersection of South Rd and E. Hill Rd</u> Problem Type: <u>Overland Erosion</u> Identification Source: <u>Allen Brook SGA</u> Ownership: <u>Private</u>		

Date of Field Data Collection: 8/10/2012

Description of Observed Conditions:
No buffer or fence between cattle grazing and stream. Evidence of direct animal access to the stream (Photo 2).

Field Photos	
	
Photo 1.	Photo 2.

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	3	2	1	No	Yes	No	No

Problem Area Data Sheet

Problem Area ID: AB-52	Latitude: 44.418945 ° N	Longitude: 73.066184 ° W
Watershed: Allen Brook <hr/> Location: East Hill Rd, 0.25 mi southeast of South Rd <hr/> Problem Type: Overland Erosion <hr/> Identification Source: Public Works Department <hr/> Ownership: Public		

Date of Field Data Collection: 8/10/2012

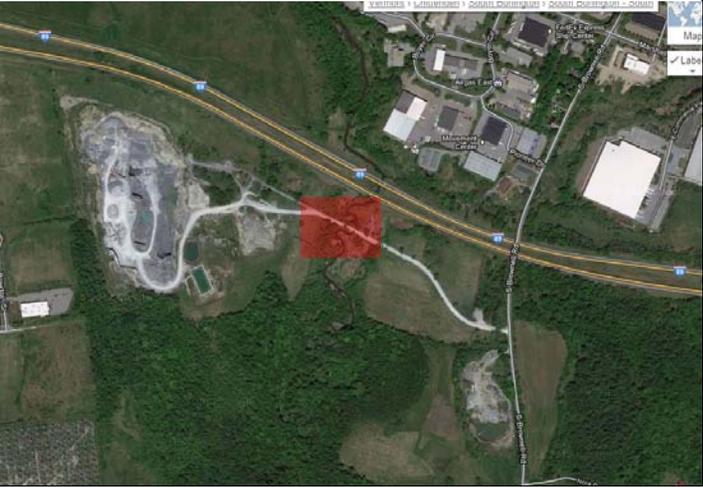
Description of Observed Conditions:
Roadside ditch is eroding/deepening over time; mostly stable now.

Field Photos	
	
Photo 1. Ditch is mostly well-vegetated	Photo 2. Bank erosion/sloughing

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impact beyond water resources?	Part of larger or systemic problem?
2	1	2	1	Yes	Yes	Yes	No

C.2. Muddy Brook

Problem Area Data Sheet

Problem Area ID: MB-02	Latitude: 44.441467° N	Longitude: 73.136081° W												
<table style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 20%; padding: 5px;">Watershed:</td> <td style="padding: 5px;">Muddy Brook</td> </tr> <tr> <td style="padding: 5px;">Location:</td> <td style="padding: 5px;">Private road off South Brownell Rd, leading to S.D. Ireland quarry</td> </tr> <tr> <td style="padding: 5px;">Problem Type:</td> <td style="padding: 5px;">Channel Erosion</td> </tr> <tr> <td style="padding: 5px;">Identification Source:</td> <td style="padding: 5px;">Muddy Brook SGA</td> </tr> <tr> <td style="padding: 5px;">Ownership:</td> <td style="padding: 5px;">Private</td> </tr> <tr> <td style="padding: 5px;">Classification Level:</td> <td style="padding: 5px;">1</td> </tr> </table>	Watershed:	Muddy Brook	Location:	Private road off South Brownell Rd, leading to S.D. Ireland quarry	Problem Type:	Channel Erosion	Identification Source:	Muddy Brook SGA	Ownership:	Private	Classification Level:	1		
Watershed:	Muddy Brook													
Location:	Private road off South Brownell Rd, leading to S.D. Ireland quarry													
Problem Type:	Channel Erosion													
Identification Source:	Muddy Brook SGA													
Ownership:	Private													
Classification Level:	1													

Date of Field Data Collection: 6/11/2012

Description of Observed Conditions:

Culvert carries Muddy Brook under haul road; haul road subject to heavy commercial truck traffic. As was noted in the SGA, this culvert is severely undersized (25% of bankfull channel width) and is causing severe scour and bank erosion downstream. Culvert bottom shows some signs of deterioration.

Field Photos



Photo 1. Scour pool at outfall

Photo 2. Bank scouring

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of a larger or systemic problem?
2	2	2	2	No	Yes	Yes	No

Relative Impact: Scour pool and bank erosion will continue to be exacerbated during rain events. Culvert is in fair to poor condition. Pro-active outreach to landowner could help ensure the replacement, when made, will be of sufficient capacity to alleviate scour and bank erosion concerns.

Problem Area Data Sheet

Problem Area ID: MB-03	Latitude: 44.452948° N	Longitude: 73.138756° W
Watershed: Muddy Brook Location: Unnamed trib joins Muddy Brook just south of Kimball Ave, between Marshall Ave and Gregory Dr. Problem Type: Channel Erosion Identification Source: Town Planning Office Ownership: Private Classification Level: 2		

Date of Field Data Collection: 6/11/2012

Description of Observed Conditions:
 Steep banks line both side of the unnamed tributary to Muddy Brook; minor erosion along toe of bank evident throughout the area. One small headcut was found in the unnamed tributary.

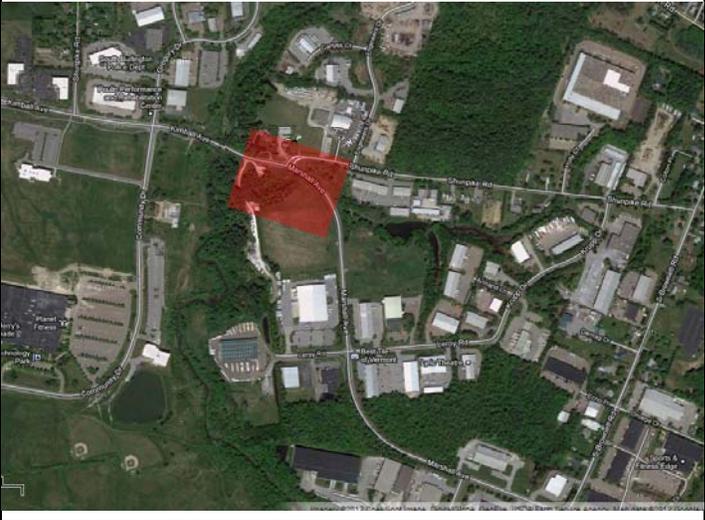
Field Photos



Photo 1. Bank erosion in tributary **Photo 2.** Steep bank leading into tributary

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of a larger or systemic problem?
2	1	1	1	No	No	No	Yes
Relative Impact: Minor, but on-going, erosion present at toe-of-slope.							

Problem Area Data Sheet

Problem Area ID: MB-06	Latitude: 44.452631° N	Longitude: 73.136370° W
Watershed: Muddy Brook Location: Unnamed trib to Muddy Brook passes under Marshall Ave near the intersection with Shunpike Rd Problem Type: Channel Erosion Identification Source: Muddy Brook SGA Ownership: Public Classification Level: 2		

Date of Field Data Collection: 6/11/2012

Description of Observed Conditions:
 36" black plastic corrugated culvert carries unnamed tributary under Marshall Ave. Culvert is undersized and is causing erosion upstream and downstream. A headcut has formed downstream of the Marshall Avenue crossing. The headcut does not appear to be migrating rapidly upslope, but increasing urbanization in the upslope watershed may make the stabilization of this feature a higher priority.

Field Photos

	
Photo 1. Existing 36" plastic culvert	Photo 2. Incised channel

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Conditions	Urgency	Impact to Public Infrastructure	Realistic to Fix	Non-stormwater related	Larger/Systemic Problem
1	1	1	1	Yes	No	No	Yes
Relative Impact: Undersized culvert and small headcut combine to increase sensitivity of this unnamed tributary to increased flows associated with development.							

Problem Area Data Sheet

Problem Area ID: MB-07	Latitude: 44.442306° N	Longitude: 73.134588° W
Watershed: Muddy Brook Location: I-89, just northwest of the So Brownell Rd crossing Problem Type: Local Drainage Identification Source: Muddy Brook SGA Ownership: Public (VTrans) Classification Level: 1		

Date of Field Data Collection: 6/11/2012

Description of Observed Conditions:

Six drains from northbound lanes and three pipes from southbound lanes of I-89 discharge directly into Muddy Brook. Southbound drains causing erosion and trenching into Muddy Brook. New riprap was recently placed around culvert in question

Field Photos



Photo 1. Recent riprap around culvert

Photo 2. Highway drainage

Prioritization Ranking Factors

Relative Impact	Frequency	Current Conditions	Urgency	Impact to Public Infrastructure	Realistic to Fix	Non-stormwater related	Larger/Systemic Problem
1	1	2	2	Yes	Yes	Yes	Yes

Relative Impact: Drainage pipes from I-89 discharge directly into Muddy Brook.

Problem Area Data Sheet

Problem Area ID: MB-08	Latitude: 44.441830° N	Longitude: 73.131161° W												
<table style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 20%;">Watershed:</td> <td>Muddy Brook</td> </tr> <tr> <td>Location:</td> <td>South Brownell Road, just north of the I-89 crossing</td> </tr> <tr> <td>Problem Type:</td> <td>Local Drainage</td> </tr> <tr> <td>Identification Source:</td> <td>Muddy Brook SGA</td> </tr> <tr> <td>Ownership:</td> <td>Public</td> </tr> <tr> <td>Classification Level:</td> <td>3</td> </tr> </table>	Watershed:	Muddy Brook	Location:	South Brownell Road, just north of the I-89 crossing	Problem Type:	Local Drainage	Identification Source:	Muddy Brook SGA	Ownership:	Public	Classification Level:	3		
Watershed:	Muddy Brook													
Location:	South Brownell Road, just north of the I-89 crossing													
Problem Type:	Local Drainage													
Identification Source:	Muddy Brook SGA													
Ownership:	Public													
Classification Level:	3													

Date of Field Data Collection: 6/11/2012

Description of Observed Conditions:

Culvert under South Brownell Road appears undersized and causes sediment and debris deposition upstream of the structure. 8-9' corrugated culvert with 4" of steady flow. Two small streams meet at mouth of culvert and take 90° turn into culvert. Pool has formed on upstream side of rock weir in stream. Wall appears to be man-made causing 18" deep pool of water with sediment build up.

Field Photos

	
Photo 1. Downstream view towards culvert	Photo 2. Upstream rock weir causing pooling

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Conditions	Urgency	Impact to Public Infrastructure	Realistic to Fix	Non-stormwater related	Larger/Systemic Problem
2	2	2	2	Yes	No	Yes	Yes
Relative Impact: Rock weir in stream causing sediment to build up. Culvert positioning not allowing for a fluid flow of either stream.							

Problem Area Data Sheet

Problem Area ID: MB-09	Latitude: 44.410058 ° N	Longitude: 73.152236 ° W
Watershed: Muddy Brook Location: Route 116/Hinesburg Rd, 0.25 mi south of Cheeseactory Rd. Problem Type: Channel Erosion Identification Source: Muddy Brook SGA Ownership: Public (VTrans) Classification Level: 1		

Date of Field Data Collection: 6/15/2012

Description of Observed Conditions:
 Stream originates in large pond/marsh area. Stream makes 90° turn before passing under bridge; minor scouring was observed at bend. Existing bridge is causing minor scour upstream of the structure. The existing bridge is 36% of bankfull channel width.

Field Photos



Photo 1. Upstream bank scouring

Photo 2. Entrance to bridge

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Conditions	Urgency	Impact to Public Infrastructure	Realistic to Fix	Non-stormwater related	Larger/Systemic Problem
1	1	2	2	Yes	No	No	No
Relative Impact: Bridge alignment appear okay; possible bank armor needed to protect against erosion.							

Problem Area Data Sheet

Problem Area ID: MB-10	Latitude: 44.445367 ° N	Longitude: 73.120968 ° W
Watershed: Muddy Brook Location: Marshall Ave. just east of Harvest Ln. Problem Type: Local Drainage Identification Source: SWMP Field Assessment Ownership: Private Classification Level: 1		

Date of Field Data Collection: 04/28/2011

Description of Observed Conditions:

Marshall Avenue and the adjacent sidewalk flood as the Muddy Brook floodplain overflows across private property. Town Public Works Department contacted the developer who cleaned the culvert; more observations are needed to ensure problem has been fully addressed.

SWMP Field Assessment Photos



Photo 1. Evidence of poor drainage

Photo 2.

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
2	2	1	2	Yes	No	Yes	Yes

Relative Impact: Intersection and roads are in floodplain for the stream.

Problem Area Data Sheet

Problem Area ID: MB-11	Latitude: 44.444799 ° N	Longitude: 73.122102 ° W
Watershed: Muddy Brook		
Location: Harvest Ln. south of Marshall Ave.		
Problem Type: Channel Erosion		
Identification Source: Muddy Brook SGA		
Ownership: Unknown		
Classification Level: 3		

Date of Field Data Collection: 6/15/2012

Description of Observed Conditions:
 Stream bed is eroding. Primary headcut measures 2.8' feet with a steady 3" flow falling. The downstream erosion is about 15' wide and extends into floodplain.

Field Photos

	
Photo 1. Bank erosion	Photo 2. Head cut

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
2	2	2	2	No	No	No	Yes
Relative Impact: Small nick-point/headcut has resulted in significant localized erosion; left unchecked erosion likely to continue to migrate upstream.							

Problem Area Data Sheet

Problem Area ID: MB-12	Latitude: 44.442633 ° N	Longitude: 73.121009 ° W
Watershed: Muddy Brook		
Location: Wal-Mart parking lot		
Problem Type: Local Drainage		
Identification Source: Town Planning Office		
Ownership: Private		
Classification Level: 2		

Date of Field Data Collection: 6/15/2012

Description of Observed Conditions:

Catch basin is slightly elevated or pavement/ground around the basin has settled and sunk in corner of parking lot. Water is flowing next to basin straight into the curb and then over and down the bank towards the Wal-Mart drive. Trenching is visible on west side of drive where water flows to. Water is seeping from hillside between two basins, unsure of this source as it had been dry for a few days before observations.

Field Photos

	
Photo 1. Water seeping from ground	Photo 2. Bank damage

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
1	2	2	2	Yes	Yes	Yes	Yes

Relative Impact: Angle of parking lot and sediment build up would suggest high storm water run-off during rain events. This can be fixed by ANR requesting that the permittee address the problem.

Problem Area Data Sheet

Problem Area ID: MB-13	Latitude: 44.440538 ° N	Longitude: 73.119519 ° W
Watershed: Muddy Brook Location: Near southwest corner of Home Depot building Problem Type: Overland Erosion Identification Source: SWMP Field Assessment Ownership: Private Classification Level: 2		

Date of Field Data Collection: 6/15/2012

Description of Observed Conditions:
 Gully erosion is occurring along the entire cut slope due, at least partially, to poor stabilization. Some are wet with minor water run-off. Rip-rap in place on bank behind Wal-Mart with a couple trees lying on their sides. Unsure if the rip rap is from original construction or a an attempt to abate on-going erosion.

Field Photos

	
Photo 1. Gullying behind Home Depot	Photo 2. Bank armor behind Wal-Mart

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
2	2	1	2	No	No	No	Yes
Relative Impact: On-going erosion and mass failure attributable to previous construction activities. This may be able to be addressed by ANR requesting that the permittee fix the problem.							

Problem Area Data Sheet

Problem Area ID: MB-14	Latitude: 44.443329 ° N	Longitude: 73.118874 ° W
Watershed: Muddy Brook Location: Harvest Lane, near entrance to Home Depot Problem Type: Erosion Identification Source: Muddy Brook SGA Ownership: Public (Town) Classification Level: 3		

Date of Field Data Collection: 6/12/2012

Description of Observed Conditions:
 Sediment built up at outfall of culvert, which is causing pooling in the culvert and very slow moving water at outfall. Large amounts of sediment downstream of culvert altering flow of water. Town Public Works Department contacted the developer who cleaned the culvert; more observations are needed to ensure problem has been fully addressed.

Field Photos



Photo 1. Looking downstream



Photo 2. Looking upstream

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
2	2	2	1	Yes	Yes	Yes	Yes
Relative Impact: Upstream erosion from Home Depot site (see MB-15) causing sediment to build up near/in culvert.							

Problem Area Data Sheet

Problem Area ID: MB-15	Latitude: 44.442133 ° N	Longitude: 73.117625 ° W
Watershed: Muddy Brook		
Location: East of the Home Depot parking lot		
Problem Type: Erosion		
Identification Source: Muddy Brook SGA		
Ownership: Private		
Classification Level: 3		

Date of Field Data Collection: 6/15/2012

Description of Observed Conditions:
 Severe bank erosion is present along most bends of reach. Major bank under cutting occurring, particularly along the bank that separates stream from Home Depot parking lot, only about 6' of bank left. Could be cause of sediment build up in MB 14, as it is just upstream of culvert.

Field Photos



Photo 1. Under cutting



Photo 2. Bank between Home Depot and Stream

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
3	2	3	3	No	Yes*	Yes	Yes

Relative Impact: Large amounts of erosion in stream causing downstream sediment build up and possible complete bank failure to Home Depot parking lot. Bank between stream and parking lot is severely eroded.

Problem Area Data Sheet

Problem Area ID: MB-18	Latitude: 44.431129 ° N	Longitude: 73.119568 ° W
Watershed: Muddy Brook Location: VT 2A, northeast of Walker Hill Rd. Problem Type: Local Drainage Identification Source: Resident Questionnaire Ownership: Public (VTrans, Town) Classification Level: 3		

Date of Field Data Collection: 6/15/2012

Description of Observed Conditions:

Majority of the issues appear to be on the southeast side of 2A; road-side ditches are eroding road shoulders. Drainage ditch crosses 2A at #3283 with 24" plastic culvert. Culvert is ½ full of sediment and debris. Hydraulic capacity of culvert decreased from aggraded material.

Field Photos



Photo 1. Aggradation in culvert.



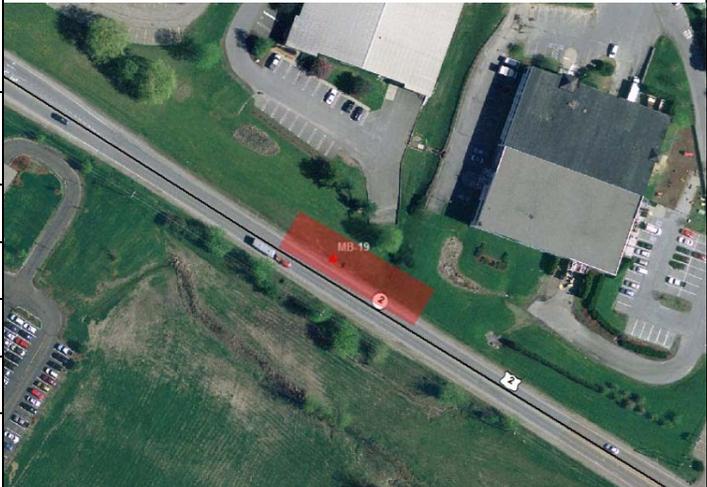
Photo 2. Road side erosion

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
2	2	2	3	Yes	Yes	Yes	Yes

Relative Impact: Drainage ditches along road needs repair; culvert needs to be cleaned out. Slope of culvert or source of backwater should be investigated to determine reason for aggradation.

Problem Area Data Sheet

Problem Area ID: MB-19	Latitude: 44.451607 ° N	Longitude: 73.115406 ° W
Watershed: Muddy Brook Location: Rt. 2 (Williston Rd) 0.1 mi. southeast of Harvest Ln Problem Type: Local Drainage Identification Source: SWMP Field Assessment Ownership: Public (VTrans, Town) Classification Level: 3		

Date of Field Data Collection: 06/15/2012

Description of Observed Conditions:

Evidence of road run-off is present on sidewalk. Trenching has formed between road and green belt.

SWMP Field Assessment Photos



Photo 1.

Photo 2.

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
1	2	2	2	Yes	Yes	No	Yes

Relative Impact: Road run-off causing erosion between road and greenbelt leading to sediment build up in sidewalk. High visibility area that would benefit from improved stormwater management.

Problem Area Data Sheet

Problem Area ID: MB-20	Latitude: 44.453336 ° N	Longitude: 73.123286 ° W
Watershed: Muddy Brook		
Location: Kirby Lane		
Problem Type: Local Drainage		
Identification Source: SWMP Questionnaire		
Ownership: Public (Town)		
Classification Level: 2		

Date of Field Data Collection: 6/15/2012

Description of Observed Conditions:
 Catch basin / dry well does not drain and causes localized flooding.

SWMP Field Assessment Photos



Photo 1. Evidence of overland flow and poor drainage



Photo 2. Standing water, as reported by local residents

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
2	2	1	1	No	Yes	Yes	Yes

Relative Impact: This is the area impacted by the Mitel toxic waste plume; it may be that the drains have intentionally not been maintained to prevent the spread of the subsurface contamination. Town Public Works Department indicates that fix will be part of planned brownfield remediation project.

Problem Area Data Sheet

Problem Area ID: MB-23	Latitude: 44.456809 ° N	Longitude: 73.118364 ° W
Watershed: Muddy Brook <hr/> Location: Lamplite Lane and White Birch Lane. <hr/> Problem Type: Local Drainage <hr/> Identification Source: Public Works Department <hr/> Ownership: Public (Town) <hr/> Classification Level: 2		

Date of Field Data Collection: 6/15/2012

Description of Observed Conditions:
 Catch basin appears to be clear of debris, difficult to determine exact cause of flooding.

Field Photos



Photo 1.

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
2	2	2	2	Yes	Yes	No	No
Relative Impact: Town Public Works Department indicates that a scoping study currently being developed and the Town has applied for EPA grant funding for design and construction.							

Problem Area Data Sheet

Problem Area ID: MB-25	Latitude: 44.472759 ° N	Longitude: 73.134958° W
Watershed: Muddy Brook Location: River Cove Road, 1.2 mi., west of VT 2A Problem Type: Erosion Identification Source: Muddy Brook SGA Ownership: Public (Town) Classification Level: 3		
Problem Description:		

Date of Field Data Collection: 6/15/2012

Description of Observed Conditions:
 Sediment build up occurring downstream of bridge. Small, 1' head cut just upstream of bridge. Signs of bank erosion on both upstream and downstream edges, majority is on west side. Existing bridge is causing significant upstream and downstream erosion; existing bridge is 52% of bankfull width.

Field Photos

	
Photo 1. Looking downstream	Photo 2. Upstream

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
1	1	2	1	Yes	No	No	No
Relative Impact: On-going in channel erosion as brook continues to adjust to increased stormwater flows and constriction caused by undersized bridge.							

Problem Area Data Sheet

Problem Area ID: MB-26	Latitude: 44.472759 ° N	Longitude: 73.134958° W
Watershed: Muddy Brook Location: SW corner of Walker Hill Rd. and Blackberry Ridge. Problem Type: Channel Erosion Identification Source: VT DEC (Pease) Ownership: Public/Private Classification Level: 3		
Problem Description:		

Date of Field Data Collection: 11/21/2012

Description of Observed Conditions:
 4-5-foot headcut forming downstream from culvert under Blackberry Ridge; streambank is actively eroding.

Field Photos

	
Photo 1. Looking upstream	Photo 2. Downstream

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
3	2	2	3	No	Yes	No	Yes
Relative Impact: Headcut will eventually move up to culvert, endangering bank stability.							

Problem Area Data Sheet

Problem Area ID: MB-27	Latitude: 44.472759 ° N	Longitude: 73.134958° W
Watershed: Muddy Brook Location: I-89 drainage, SW of Wal-Mart Problem Type: Channel Erosion Identification Source: VT DEC (Pease) Ownership: Public (VTrans)/Private Classification Level: 2		
Problem Description:		

Date of Field Data Collection: 12/21/2012

Description of Observed Conditions:
 Streambank erosion downstream of culvert. Access to floodplain ~100ft downstream.

Field Photos



Photo 1. Looking upstream at culvert under I-89 N.Bound

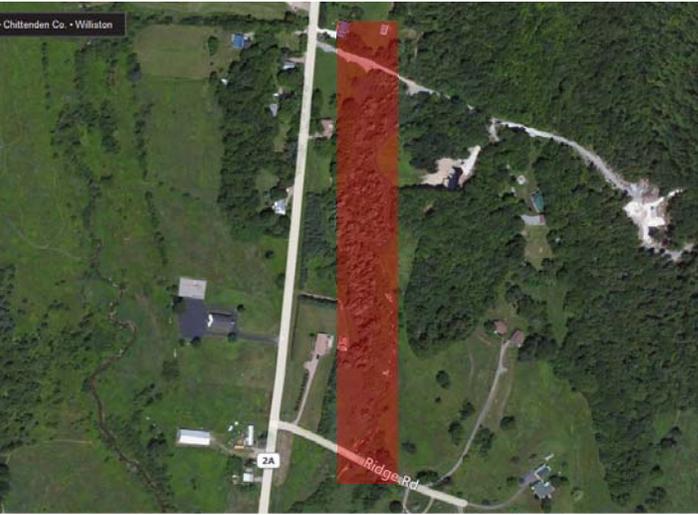


Photo 2. Looking downstream from culvert under I-89.

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of larger systemic problem?
2	1	2	2	No	No	No	Yes
Relative Impact: Some stabilization measures have been implemented. Rip-rap has failed in places and bank erosion continues.							

C.3. Sucker Brook

Problem Area Data Sheet

Problem Area ID: SB-01	Latitude: 44.405225° N	Longitude: 73.125938° W
Watershed: Sucker Brook Location: From where Sucker Brook crosses under Route 2A south of Ridge Rd, north ~1500 feet Problem Type: Channel Erosion Identification Source: Sucker Brook SGA Ownership: Public (VTrans, Town)/Private Classification Level: 3		

Date of Field Data Collection: 6/11/2012

Description of Observed Conditions:

A series of small headcuts (6-10" in height) and scour pools are visible in this section of Sucker Brook. As was noted in the 2007 SGA, and reconfirmed during this process, the culverts found in this reach are all undersized, especially the two which take the stream under Ridge Road (5.5') and the driveway crossing at the reach break (4.5'). Channel width through this area ranges between 13 and 15 feet.

Field Photos



Photo 1. Culvert under Ridge Road

Photo 2. Culvert under private drive

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of a larger or systemic problem?
2	2	2	1	Yes	Yes	Yes	Yes

Relative Impact: The severe aggradation of coarse material in this stream reach has been problematic for the Town and the landowners whose driveways and roads cross the channel. Installation of larger culverts to alleviate the periodic upstream flooding around these structures, and allow the passage of the sediment through the reach.

Problem Area Data Sheet

Problem Area ID: SB-09	Latitude: 44.422601° N	Longitude: 73.113565° W
Watershed: Sucker Brook Location: Along Old Creamery Road, from Oak Knoll Road to Lyman Drive Problem Type: Erosion Identification Source: Sucker Brook SGA Ownership: Private Classification Level: 3		

Date of Field Data Collection: 6/11/2012

Description of Observed Conditions:

In this area, Sucker Brook is being impacted by residential encroachment (and the attendant stormwater runoff) and unnatural structures placed in and across the channel. Specific observed problems include eroding streambanks and channel incision.

Field Photos



Photo 1. Bank erosion in channel



Photo 2. Foot bridge and "abutment"

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of a larger or systemic problem?
3	2	2	2	No	Yes	No	Yes

Relative Impact: Relatively new development upstream is likely to have increased hydraulic loading; promoting practices to keep stormwater on-site, limit channel encroachments and increase woody vegetation in the buffer may help reduce in-channel erosion.

Problem Area Data Sheet

Problem Area ID: SB-12	Latitude: 44.403875° N	Longitude: 73.130124° W
Watershed: Sucker Brook Location: Near intersection of Chaloux Lane with Lincoln Road Problem Type: Local Drainage Identification Source: Public Works Ownership: Public (Town)/Private Classification Level: 3		

Date of Field Data Collection: 6/11/2012

Description of Observed Conditions:

Culverts running under Chaloux Ln and private drive are significantly overgrown. Some sediment build up is present at outfalls. Northeast corner of intersection showing signs of erosion related to road run-off. Southern edge of Lincoln Road also showing signs of run-off and trenching.

Field Photos



Photo 1. Road erosion

Photo 2. Culvert under Chaloux Lane

Prioritization Ranking Factors

Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of a larger or systemic problem?
2	2	2	2	Yes	Yes	No	Yes

Relative Impact: Current drainage patterns lead to erosion during moderate and large runoff events.

Problem Area Data Sheet

Problem Area ID: SB-13	Latitude: 44.399239° N	Longitude: 73.114895° W
Watershed: Sucker Brook <hr/> Location: Butternut Road, approximately ½ mile east of intersection with Route 2A <hr/> Problem Type: Erosion <hr/> Identification Source: Public Works <hr/> Ownership: Public (Town) <hr/> Classification Level: 1		

Date of Field Data Collection: 6/11/2012

Description of Observed Conditions:

Retaining wall is collapsing; guardrails are leaning as well. Wall is made of 2'x5' concrete blocks, stacked two high, running 17 columns long. Public Works is aware of the concern and has indicated plans to reinstall the retaining wall in 2013. Depending on where things stand in the design and funding of the fix, may have an opportunity to consider a less structural solution.

Field Photos



Photo 1. Retaining wall (looking upstream)



Photo 2.

Prioritization Ranking Factors							
Relative Impact	Frequency	Current Condition	Urgency	Impact to public infrastructure?	Realistic to fix?	Impacts beyond water resources?	Part of a larger or systemic problem?
1	2	3	3	Yes	Yes	Yes	No
Relative Impact: Failing retaining wall is impacting road surface and exacerbating erosion along road.							

C.4. Winooski River

Problem Area Data Sheet

Problem Area ID: WR-01	Latitude: 73.117346 N	Longitude: 44.475213 W
Watershed: <u>Winooski River</u>		
Location: <u>Shirley Circle</u>		
Problem Type: <u>Local Drainage</u>		
Identification Source: <u>SWMP Field Assessment</u>		
Ownership: <u>Private/Public (Town)</u>		
Classification Level: <u>3</u>		

Date of Field Data Collection: 05/14/2012

Description of Observed Conditions:

Heavy erosion, significant ruts and mud holes forming in road. Run off from neighboring lots on Shirley Circle and James Brown Dr exceeds the capacity of the ditch beside Shirley Circle and washes out the roadway. Private section of road that is over utilized by large vehicles from Town Garage located at end of public road.

Field Photos



Photo 1. Roadway washout.



Photo 2. Primary source of runoff.

Relative Impact	Frequency	Current Conditions	Urgency	Impact to Public Infrastructure	Realistic to Fix	Non-stormwater related	Larger/Systemic Problem
3	3	3	3	No	Yes	No	Yes

Relative Impact: Heavy erosion in and along road; significant mud ruts and potholes abound. Evidence of sediment deposition in the area around the discharge pipe.

Problem Area Data Sheet

Problem Area ID: WR-02	Latitude: 73.112286 N	Longitude: 44.471829 W												
<table style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 20%; padding: 5px;">Watershed:</td> <td style="padding: 5px;">Winooski River</td> </tr> <tr> <td style="padding: 5px;">Location:</td> <td style="padding: 5px;">Route 2A, just south of the intersection with River Cove Rd</td> </tr> <tr> <td style="padding: 5px;">Problem Type:</td> <td style="padding: 5px;">Local Drainage</td> </tr> <tr> <td style="padding: 5px;">Identification Source:</td> <td style="padding: 5px;">Reported by resident</td> </tr> <tr> <td style="padding: 5px;">Ownership:</td> <td style="padding: 5px;">Private</td> </tr> <tr> <td style="padding: 5px;">Classification Level:</td> <td style="padding: 5px;">1</td> </tr> </table>	Watershed:	Winooski River	Location:	Route 2A, just south of the intersection with River Cove Rd	Problem Type:	Local Drainage	Identification Source:	Reported by resident	Ownership:	Private	Classification Level:	1		
Watershed:	Winooski River													
Location:	Route 2A, just south of the intersection with River Cove Rd													
Problem Type:	Local Drainage													
Identification Source:	Reported by resident													
Ownership:	Private													
Classification Level:	1													

Date of Field Data Collection: 5/14/2012

Description of Observed Conditions:

New stone driveway in place. New 12" corrugated plastic culvert installed with ½" of flow moving from wetland adjacent to drive into what appears to be a 6" plastic pipe running underground into the woods. May be an issue if large amounts of water begin to flow through culvert. Second culvert is new 8" corrugated plastic helping to ease drainage. Some iron staining present. Disturbed areas have not been fully stabilized with seed and mulch.

Field Photos

	
Photo 1. New 12" culvert flowing into small 6" pipe	Photo 2. New culverts

Relative Impact	Frequency	Current Conditions	Urgency	Impact to Public Infrastructure	Realistic to Fix	Non-stormwater related	Larger/Systemic Problem
1	2	1	1	No	No	No	No
Relative Impact: Localized erosion attributable to poor site stabilization; larger culvert likely to address original concern.							

Problem Area Data Sheet

Problem Area ID: WR-03	Latitude: 73.044511 N	Longitude: 44.466002 W
Watershed: Winooski River		
Location: Chapman Lane, about 400' east of North Williston Rd		
Problem Type: Channel Erosion		
Identification Source: SWMP Field Assessment		
Ownership: Private (railroad)		
Classification Level: 1		

Date of Field Data Collection: 05/14/2012

Description of Observed Conditions:
 Undercutting is occurring downstream of the box culvert under the railroad bed, including trees on NW side of bank. 1-2" of flow moving through with 6" drop at headcut.

Field Photos

	
Photo 1. Headcut at the outlet of box culvert under railroad bed.	Photo 2. South bank undercutting

Relative Impact	Frequency	Current Conditions	Urgency	Impact to Public Infrastructure	Realistic to Fix	Non-stormwater related	Larger/Systemic Problem
3	2	2	2	No	Yes	Yes	No
Relative Impact: Bank erosion present, and will likely continue if left unchecked; culvert may be or become a barrier to aquatic organism passage if the headcut continues to deepen.							

Problem Area Data Sheet

Problem Area ID: WR-04	Latitude: 73.034995 N	Longitude: 44.462254 W
Watershed: <u>Winooski River</u>		
Location: <u>Private drive off the end of Chapman Lane</u>		
Problem Type: <u>Local Drainage</u>		
Identification Source: <u>SWMP Field Assessment</u>		
Ownership: <u>Private</u>		
Classification Level: <u>1</u>		

Date of Field Data Collection: 05/14/2012

Description of Observed Conditions:
 Stream running parallel to farm road on south side. Road on both sides of existing culvert lower, causing drainage issue and ponding. Attempts made by landowner to channel water off of road do not seem to be working.

Field Photos



Photo 1. Straightened stream flowing to culvert **Photo 2.** Erosion occurring at the discharge point.

Relative Impact	Frequency	Current Conditions	Urgency	Impact to Public Infrastructure	Realistic to Fix	Non-stormwater related	Larger/Systemic Problem
3	3	3	2	No	Yes	No	Yes

Relative Impact: Water is channeling along road and ponding in middle of road at crossing; significant amount of sediment deposited along/ in stream.

Problem Area Data Sheet

Problem Area ID: WR-05	Latitude: 73.051618 N	Longitude: 44.462005 W
Watershed: Winooski River		
Road Name: North Williston Rd, just east of Fontaine Ln		
Problem Type: Overland Erosion		
Identification Source: SWMP Field Assessment		
Ownership: Public (Town)		
Classification Level: 3		

Date of Field Data Collection: 05/14/2012

Description of Observed Conditions:

Erosion occurring on both sides of No. Williston Road near intersection with Fontaine Lane, due in part to heavy weight vehicles turning in/out. Ditches are eroding across from drive for #1923/1925.

Field Photos



Photo 1. Road decay along No. Williston Road

Photo 2. Shoulder erosion, sediment deposition in ditch

Relative Impact	Frequency	Current Conditions	Urgency	Impact to Public Infrastructure	Realistic to Fix	Non-stormwater related	Larger/Systemic Problem
2	2	2	2	Yes	Yes	Yes	No

Relative Impact: Recent armoring is apparent in a few spots, including around the culvert; unlikely to be sufficient as long-term solution

Problem Area Data Sheet

Problem Area ID: WR-06	Latitude: 73.031686 N	Longitude: 44.445523 W
Watershed: Winooski River <hr/> Road Name: Gov Chittenden Rd, about 1.5 miles from the intersection with No. Williston Rd <hr/> Problem Type: Local Drainage <hr/> Identification Source: SWMP Field Assessment <hr/> Ownership: Public (Town) <hr/> Classification Level: 3		

Date of Field Data Collection: 05/12/2012

Description of Observed Conditions:

Class 4 road closed Nov 1-May1. Lack of roadside conveyance facilities and proper road crown/cross slope is causing runoff to erode the dirt/gravel roadway. Multiple in-road gullies beginning to form, as road is not crowned/sloped so as to reliably shed water. Undermining of tree and roots along north-side of road.

Field Photos



Photo 1. Erosion in the roadway.

Photo 2. Sediment discharge along road shoulder.

Relative Impact	Frequency	Current Conditions	Urgency	Impact to Public Infrastructure	Realistic to Fix	Non-stormwater related	Larger/Systemic Problem
2	2	2	1	Yes	Yes	No	No

Relative Impact: Some sediment movement but Class IV road is closed in winter; Town may want to be wary about precedent of fixing Class IV road if it decides to pursue this project.

Problem Area Data Sheet

Problem Area ID: WR-08	Latitude: 73.045167 N	Longitude: 44.435489 W
Watershed: Winooski River		
Road Name: Sunrise Circle, about 500' north of Route 2		
Problem Type: Local Drainage		
Identification Source: Public Works		
Ownership: Public (Town)		
Classification Level: 3		

Date of Field Data Collection: 5/14/2012

Description of Observed Conditions:

Catch basins positioned in lawns away from road. Storm water flows down edge of road causing gullying and road decay. Town remedy of rubber, semi permeable asphalt working in areas it was installed but it appears that the material was not extended all of the way downhill. All storm water flows to catch basin at center of circle, causing some road damage. Water drains from the catch basin via an 8" corrugated metal pipe which discharges to a drainage ditch in the back of development.

Field Photos



Photo 1. End of town remedy, beginning of erosion



Photo 2. Soil/Rock depositing at "mid-circle" catch basin

Relative Impact	Frequency	Current Conditions	Urgency	Impact to Public Infrastructure	Realistic to Fix	Non-stormwater related	Larger/Systemic Problem
2	2	2	2	Yes	Yes	Yes	No

Relative Impact: High amounts of sediment moving downhill causing washouts; little evidence of sediment deposits at outfall pipe suggesting that much of the material may be settling out in the mid-circle yard.

**APPENDIX D: ANALYSIS OF CURRENT AND BUILD-OUT
STORMWATER DESIGN FLOW FOR MUDDY BROOK**

November 14, 2012

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To: Jessica Andreoletti
Town of Williston
7900 Williston Road
Williston, VT 05495

MEMO



STONE ENVIRONMENTAL INC

From: Julie Moore
Direct Phone: 802-229-1881
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535 Stone Cutters Way
Montpelier, Vermont
05602 USA

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SEI No. 12-055
Re: Current and Build-Out Stormwater Design Flow Analysis for Muddy Brook

1.1 Develop Analysis Unit Sub-Watersheds

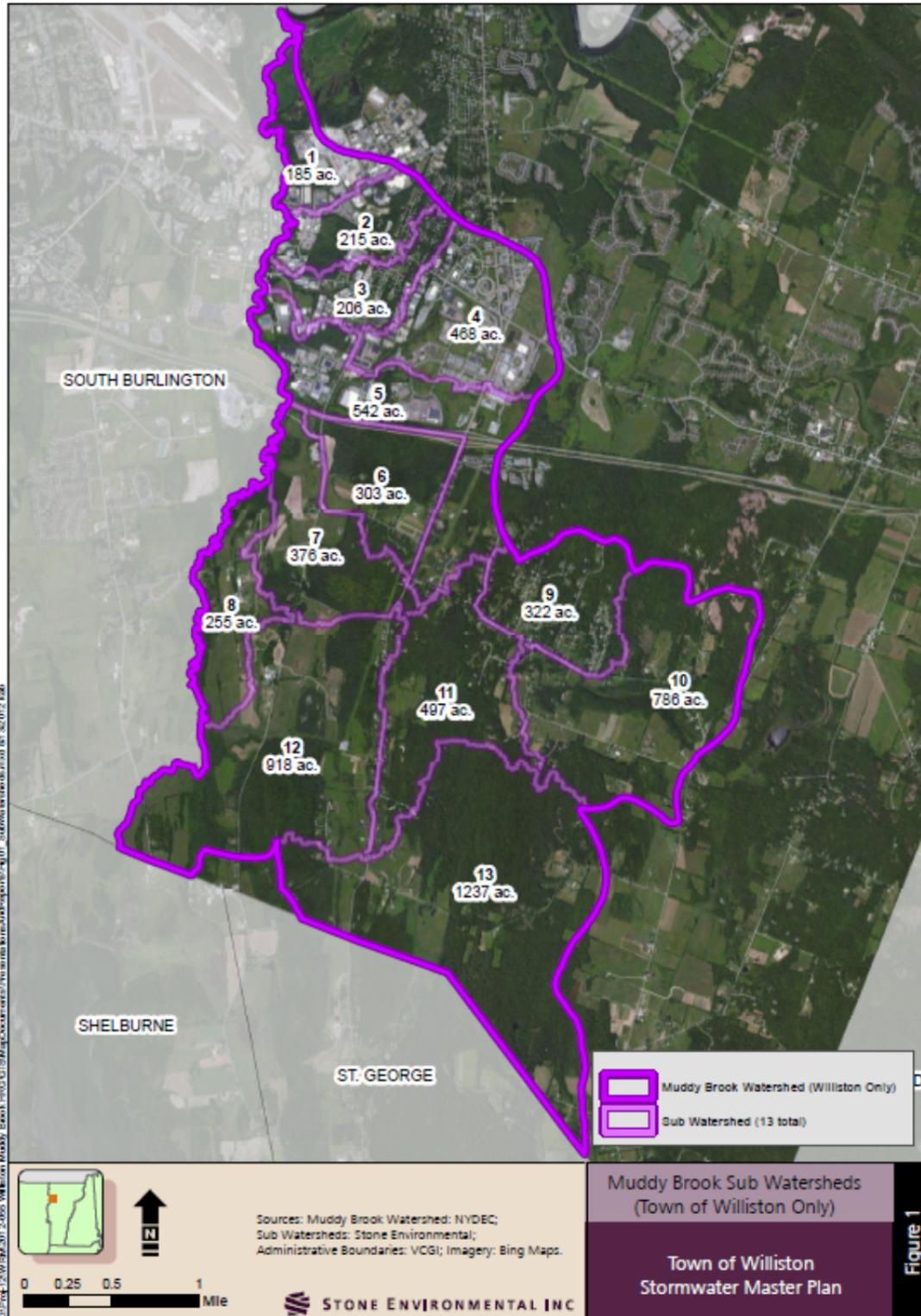
The current condition and build-out stormwater design flows were summarized at the sub-watershed level for the Muddy Brook watershed within the Town of Williston. Sub-watersheds were delineated using the 1.5 meter resolution 2004 LiDAR digital elevation model (DEM). Esri ArcGIS v.10 Hydro tools were used to delineate watersheds with at least 200 acres of flow. The resulting watersheds were then edited based on stormwater infrastructure and road features in ArcGIS. The final sub-watershed dataset includes 13 sub-watersheds within the Muddy Brook watershed in the Town of Williston. See Figure 1 for the sub-watersheds that are used for summarizing current condition and build-out condition stormwater design flows.

1.2 Evaluate Current Development and Stormwater Conditions

It is important to understand baseline environmental and development conditions and how those conditions relate to current land cover and effective impervious area in order to estimate stormwater runoff in the target subwatersheds, before assessing how stormwater flows may increase under future development patterns. Stone has completed a current condition stormwater peak flow analysis using the Rational Method.

1.2.1 Estimating Mapped Impervious Area at Current Condition

The first step in estimating stormwater runoff and design flow at the current condition was to estimate mapped impervious surface and effective impervious surface for sub-watersheds. Effective impervious area recognizes that some impervious areas are completely surrounded by pervious areas and therefore have less of an impact on aquatic ecosystems. “Effective impervious area” is basically the impervious cover that provides stormwater flows fairly directly and quickly to streams.



A statewide impervious surface dataset has just been completed by Vermont Department of Environmental Conservation (VTDEC), based on 2008 high resolution near infrared National Agriculture Imagery Program (NAIP) imagery. Stone reviewed the statewide dataset within the study area and made corrections based on various imagery sources, including Esri, Bing Maps, and Vermont Center for Geographic Information (VCGI). Examples of corrections included areas where there was a mis-classification or areas where development has occurred since 2008. The finalized impervious surface layer served as the current condition mapped impervious area (MIA) data layer. The MIA was summarized by sub-watershed.

1.2.2 Estimating EIA at Current Condition

Effective impervious surface (EIA) area was calculated for each sub-watershed based on the methodology outlined by Sutherland (2005, http://pacificwr.com/Publications/Estimating_EIA.pdf). First, sub-watersheds were classified based on the type and amount of stormwater management infrastructure. There are five potential classes outlined by Sutherland including: 1) average basins, where the drainage is predominated by storm sewers, and curb and gutter, although rooftops are not connected to the storm sewer; 2) highly connected basins, where residential rooftops are connected to the storm sewer system; 3) totally connected basins, where 100% of the urbanized area within the basin is storm sewered and all impervious surfaces are directly connected; 4) somewhat disconnected basins, where at least 50% of the developed area are served by grassy swales or roadside ditches; and, 5) extremely disconnected basins, where runoff from a large portion (70%+) of the basin is not collected. See Figure 2 for the EIA classification by sub-watershed.

1.2.3 Estimating Stormwater Peak Flow at Current Condition

Hydrologic conditions were analyzed using the Rational Method (Equation 1). The Rational Method is the simplest technique used to calculate peak stormwater runoff flow rates. It is commonly used to design storm sewer systems for catchments up to 160 acres and to predict storm flows for watersheds up to 20mi² (Gupta 2008, p.729). The Williston portion of the Muddy Brook watershed is approximately 9.8 mi², and we believe the resulting outputs from calculations are valid for watershed planning purposes. The calculated values should be used to compare relative flow and CxA product values as explained later. At this scale the Rational Method is not expected to provide design-level estimates for flow rates.

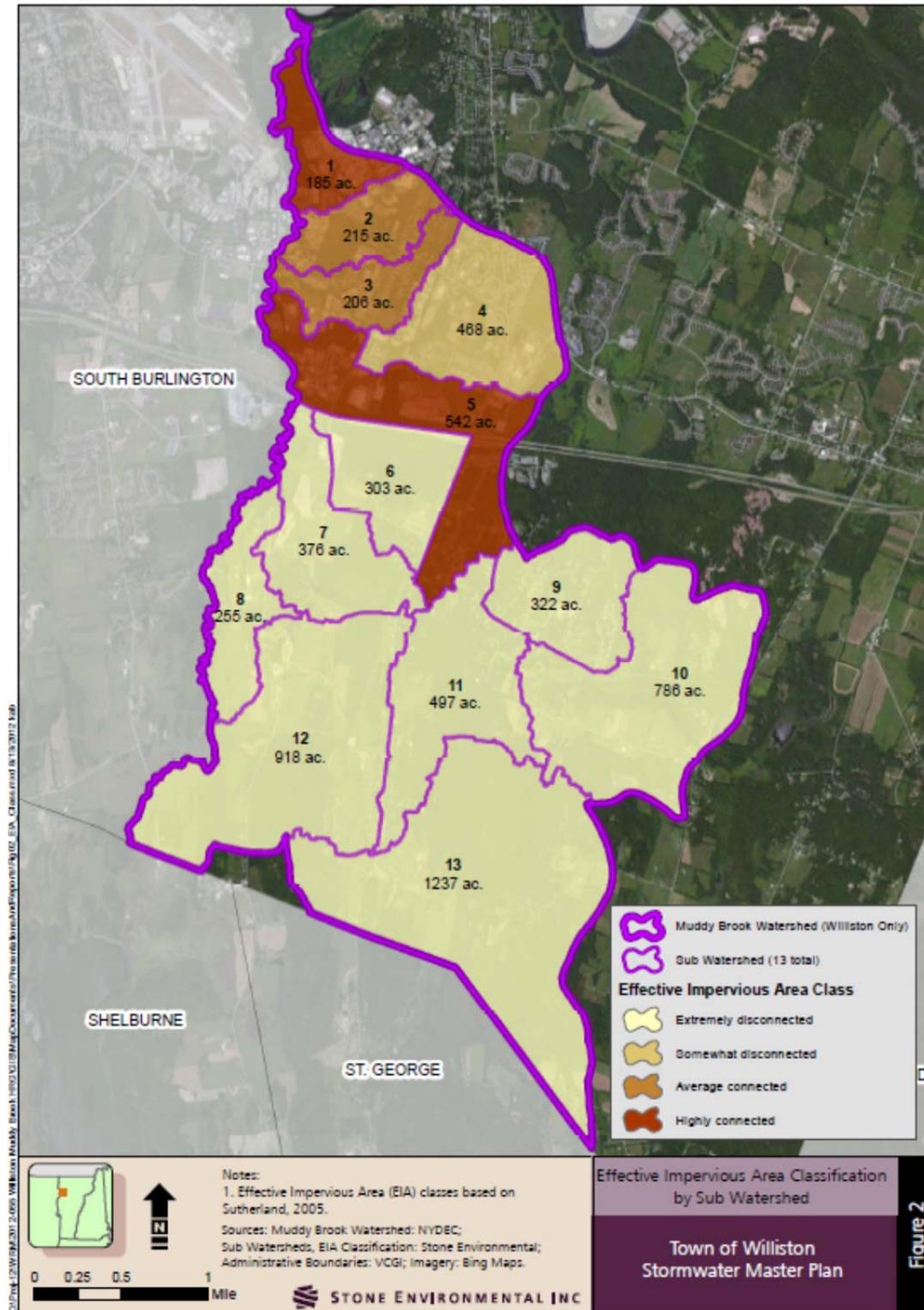
$$Q = CiA \quad (\text{Equation 1})$$

Where: C = Runoff Coefficient; i = intensity; A = Area

The unit-less runoff coefficient (C) was selected based on type of land cover. For example, assigning impervious areas a C-value of 0.9 and forested land a C-value of 0.18. For the Muddy Brook watershed, the EIA of each catchment was assigned a C-value of 0.9 for the EIA; a composite value was determined for the remaining land. This value is a weighted average based on the type of land cover and the respective amount of the catchment area in each classification using Equation 2.

$$C = \frac{\sum(C_i \times A_i)}{\sum A_i} \quad (\text{Equation 2})$$

This composite C-value was then applied to the non-EIA portion of each sub-watershed catchment.



The rainfall intensity (i) is selected based on the time to concentration for the watershed. The time to concentration is the time it would take a water drop that falls on the most remote portion of a watershed to reach the mouth of the receiving stream. This value depends on the slope of the land, type of land cover, channel velocity, and other factors. These factors are all highly variable, so a precise value is not realistic; this is one of the reasons the Rational Method is not used for design in large catchment areas. In the Rational Method the time to concentration is assumed to equal the duration of the highest intensity storm-event for a watershed. For the Williston portion of the Muddy Brook watershed this was approximately 140 minutes. Estimated rainfall intensities for 2-yr, 10-yr, 25-yr, and 50-yr return rates were taken from the *Rainfall Frequency Atlas of the United States* for 120 and 180 minute intervals and interpolated at 140 minutes. (http://www.nws.noaa.gov/oh/hdsc/PF_documents/TechnicalPaper_No40.pdf).

The EIA, value as described in Section 1.2.2, was used for area (A) values to represent impervious land cover. The A -value for the remaining portion of the watershed was given by subtracting EIA from the total area for each sub-watershed catchment (Equation 3).

$$A = A_{total} - EIA \text{ (Equation 3)}$$

To compare current conditions to predicted build-out conditions, the respective current and build-out values for EIA were used for *Area* (A) in Equation 1.

1.3 Evaluate Design Flow for Build-Out

Stone used a residential zoning build-out analysis developed by the Town of Williston, a commercial zoning build-out analysis developed by Stone, and the current condition stormwater analysis results in conducting the build-out condition stormwater flow analysis. The analysis estimates stormwater runoff delivered to each receiving stream at the build-out condition based on current development, current zoning regulations, and projected growth under the 2-year, 10-year, 25-year, and 100-year design storm conditions. This evaluation and the predicted increases in stormwater runoff rates are a worst case scenario, in that it was assumed none of the new development that occurred would be subject to a state stormwater discharge permit. Although the majority of the developable parcels in Williston are larger than one-acre, it would be difficult to predict just how the development will unfold. As such, we decided to consider the worst case scenario where future development would occur absent stormwater controls.

1.3.1 Build-Out Analysis and Estimating MIA at Build-Out

The Town of Williston build-out analysis resulted in an estimate of additional build-out dwelling units per parcel for **residential** zoning classes. Stone conducted a separate build-out analysis for **commercial** and **industrial** zoning classes within the Muddy Brook watershed, based on guidance from the Town of Williston. The results of these analyses were used in estimating MIA at build-out.

1.3.1.1 Residential Zoning Classes

The Town of Williston build-out analysis resulted in an estimate of additional dwelling units at build-out (attribute 'pot_du') for parcels within the residential zoning classes. This estimated number of additional

dwelling units from the Town of Williston build-out analysis was used in estimating MIA for residential zoning class parcels. The residential zoning classes within the Muddy Brook watershed include the Agriculture/Rural Residential Zoning District (ARZD) and the Residential Zoning District (RZD). The build-out analysis took environmental and development setbacks into consideration to determine an appropriate number of additional dwelling units at build-out for these zoning classes.

To estimate the impervious surface for each additional dwelling unit, an average, per dwelling unit impervious area (acres) was calculated by zoning class, based on the current condition mapped impervious surface, within the Muddy Brook watershed. To calculate the average ‘per dwelling unit’ impervious surface acreage, the current condition mapped impervious surface layer (see Section 1.2.1 above) was intersected with parcels to determine the per parcel impervious surface area at current condition. The current number of dwelling units on each parcel was determined from the Town of Williston build-out analysis for parcels in residential zoning classes (attribute ‘Num_Dwell’). The mapped impervious surface acreage at current condition was divided by the current number of dwelling units by parcel and averaged by zoning class. Table 1 gives the resulting average area of impervious surface per dwelling unit by zoning class.

Table 1. Average per dwelling unit impervious surface area at current condition by zoning class.

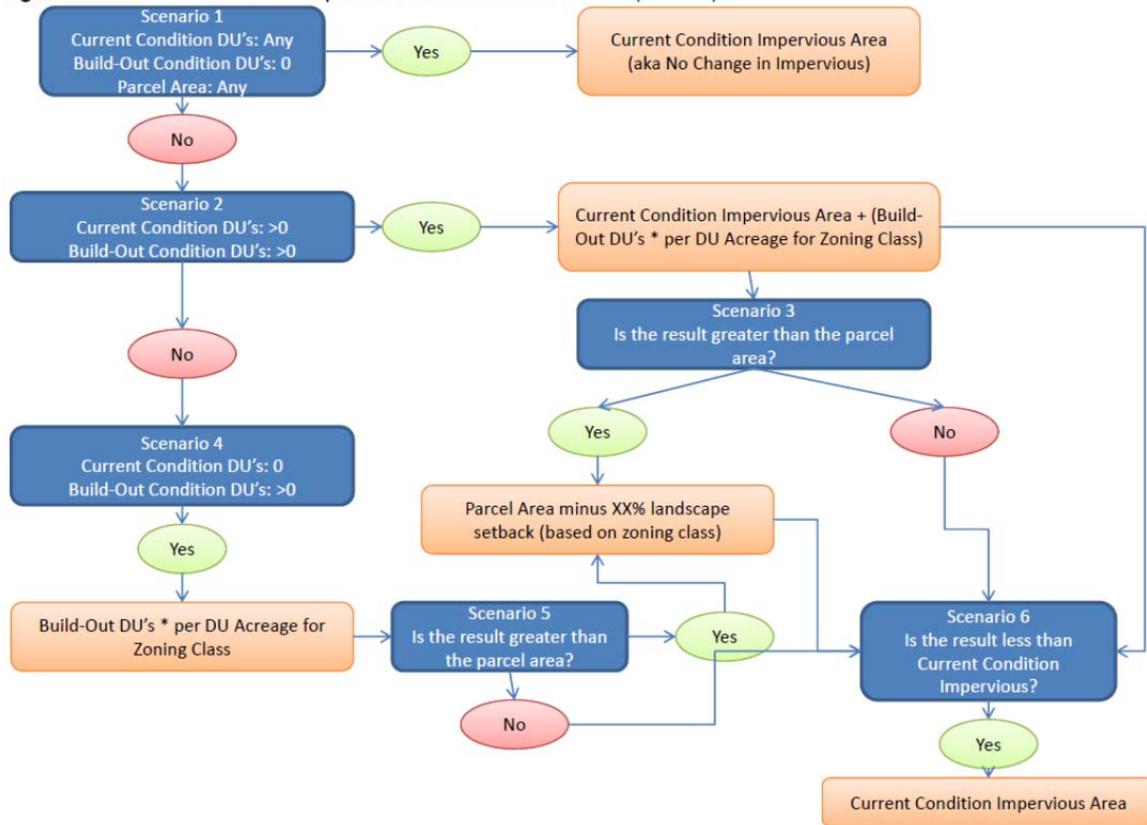
District	DistrictAb	Average Impervious Acreage Per Dwelling Unit at Current Condition	Number of Parcels (>0 DUs) at Current Condition
Agriculture/Rural Residential Zoning District	ARZD	0.242	451
Residential Zoning District	RZD	0.137	118

The estimated impervious surface area at build-out was calculated for each residential parcel. Table 2, below, provides the calculation based on the number of current condition dwelling units, the estimated build-out dwelling units, and parcel area as it relates to estimated build-out impervious surface. Also, see Figure 3 for a diagram of the scenario decision tree. The ‘CC Dwelling Units’ refers to the number of dwelling units at current condition, based on the ‘Num_Dwell’ attribute in the Town of Williston build-out analysis for parcels in residential zoning classes. The ‘BO Dwelling Units’ refers to the number of additional dwelling units at build-out, based on the ‘pot_du’ attribute in the Town of Williston build-out analysis for parcels in residential zoning classes.

For Scenario 1, if a parcel has no additional dwelling units at build-out, it is assumed that there is no change in impervious surface from current condition to build-out and the impervious surface acreage at build-out will be the same as the current condition.

For Scenario 2, for parcels that have greater than zero dwelling units at current condition and greater than zero additional dwelling units at build-out, the impervious surface acreage at current condition will be added to an estimated additional impervious surface acreage based on the number of additional dwelling units and the zoning-based impervious acreage per dwelling unit (see Table 1 above).

Figure 3. Per Parcel Build-Out Impervious Surface Decision Tree (Table 2)



There may be cases where the resulting impervious surface at build-out from Scenario 2 will result in an acreage that is greater than the parcel area. This may occur if the current condition impervious surface area is particularly high or if the per dwelling unit estimate of impervious surface from Table 1 is too high. An example of when this estimate may be too high is if the new development at build-out is assumed to be multi-story and the current condition development within the zoning class is in the form of traditional single family homes. If the resulting impervious surface area at build-out for Scenario 2 is greater than the parcel area, the total impervious surface area for that parcel will be scaled back to the Parcel Area minus an assumed, zoning-based, minimum landscaping setback. This is Scenario 3. Estimated impervious cover under build-out conditions for each zoning district is presented in Table 2 below.

Table 2. Estimated Impervious Cover, Under Build-Out Conditions.

	Impervious surface limited by density		Impervious surface limited by setbacks/landscape buffers	
Zoning District	Max % imp	Density Description	Max % imp	Setback Description
ARZD	13%	<ul style="list-style-type: none"> • Average density = 1DU/80,000 SF; 1DU/1.84 Acres • 25 ac = 1,088,975 SF • Allowable density = 1,088,975 SF / 80,000 SF = 13 DU (<i>round down</i>) • 13 DU x .242 imp/DU* = 3.146 imp • 3.146 imp of 25 ac = 13% imp and 87% green 	N/A	Average Side/Rear = 15 ft Road Estimate = 37 ft 83% imp and 17% green
RZD	41%	<ul style="list-style-type: none"> • Average Density = 3DU/Acre; 1DU/.33 ac (14,520 SF) • 5 ac = 217,795 SF • 5 ac/.33 ac = 15 DU (<i>round down</i>) • 15 DU x .137 imp/DU = 2.055 imp • 2.055 imp of 5 ac = 41% imp and 59% green 	N/A	Average Side/Rear = 12.5 ft Road Estimate = 37 ft 85% imp and 15% green
VZD	27%	<ul style="list-style-type: none"> • Average density = 2DU/ac; 1DU/.50 ac (21,779 SF) • 5 ac = 217,795 SF • 5 ac/.5 ac = 10 DU (<i>round down</i>) • 10 DU x .137 imp/DU = 1.37 imp • 1.37 imp of 5 ac = 27% imp and 73% green 	N/A	Average Side/Rear = 15 ft Road Estimate = 37 ft 83% imp and 17% green
Mixed-Use/Commercial MURZD, MUCZD, TCZD, BPZD, GZDN, GZDS	N/A	While residential uses are allowed in the MURZD, MUCZD, and TCZD, impervious area should be calculated based on required landscaping and road setbacks. Dwellings will be situated in multi-family structures and above retail/commercial space.	85.5%	Average Side/Rear = 11 ft (13+9/2) Road Estimate = 37 ft ** 83% imp and 14.5% green
Industrial IZDE, IZDW	N/A	Residential not allowed.	83%	Average Side/Rear = 15 ft (23+13+9/3) Road Estimate = 37 ft 83% imp and 17% green

For Scenario 4, for parcels that have zero dwelling units at current condition and greater than zero additional dwelling units at build-out, an estimated impervious surface acreage based on the number of dwelling units at build-out and the zoning-based impervious acreage per dwelling unit (see Table 1 above) is used. The current condition impervious area will be ignored, because it is assumed that the parcels are either undeveloped at current condition, that there will be a change of use from commercial/industrial development to residential development, or that residential development will occur above commercial development.

Scenario 5 is similar to Scenario 3, where the impervious surface estimate at build-out will be scaled back to the Parcel Area minus an assumed, zoning-based landscaping setback, in cases where the results of Scenario 4 are greater than the parcel area. The maximum impervious cover for the ARZD class is 13% and the maximum impervious cover for the RZD class is 41%.

There may be cases where the estimated impervious surface area at build-out for a parcel is less than the current condition impervious surface area, for Scenarios 2 through 5. It is assumed that impervious surface will not decrease over time. For this reason, any parcel where the estimated impervious surface at build-out is less than the calculated impervious surface at current condition, the current condition impervious surface area will be used (Scenario 6). This approach, summarized in Table 3, can be used to calculate the BOI – which is the MIA at build-out for residential parcels.

Table 3. Impervious surface calculations based on the number of current condition dwelling units, estimated build-out dwelling units, and parcel area as it relates to estimated build-out impervious area.

Scenario	CC Dwelling Units	BO Dwelling Units	Parcel Area	BO Impervious Calculation (BOI)
1	Any	0	Any	CCI
2	>0	>0	> BOI	CCI + (BODU * BODU_Area)
3	>0	>0	< BOI	Parcel Area – Zoning-Based Landscaping Setback
4	0	>0	> BOI	BODU * BODU_Area
5	0	>0	< BOI	Parcel Area – Zoning-Based Landscaping Setback
6*	Any	>0	> BOI	CCI

* In cases where estimated BOI < CCI

Where: CCI = Current Condition Impervious Area in Acres
BOI = Total Build-Out Impervious Area in Acres
BODU = Build-Out Dwelling Units
BODU_Area = Assumed Per Dwelling Unit Impervious Area in Acres (by zoning class)

1.3.1.2 Commercial and Industrial Zoning Classes

Stone conducted a separate build-out analysis for **commercial** and **industrial** zoning classes within the Muddy Brook watershed, based on guidance from the Town of Williston. The commercial and industrial build-out analysis included parcels in the following zoning classes:

1. Business Park Zoning District (BPZD),
2. Gateway Zoning District North (GZDN), – NOT IN STUDY AREA, not included in analysis
3. Gateway South Zoning District (GZDS),
4. Mixed-Use Commercial Zoning District (MUCZD),

5. Mixed Use – Residential Zoning District (MURZD),
6. Industrial Zoning District East (IZDE),
7. Industrial Zoning District West (IZDW). – NOT IN STUDY AREA, not included in analysis
8. Taft Corners Downtown Zoning District (TCZD)

In the commercial and industrial zoning classes, a GIS analysis was used to remove land constraints (wetlands, floodplains, surface water, slope (>30%), road setbacks, conserved lands) from Williston parcels, in addition to a zoning specific landscaping setback. Sources for land constraint datasets are listed below.

1. Wetlands, floodplains, surface water, slope, state conserved lands: VCGI
2. Road setbacks (as described in Table 4, below), town conserved lands: Town of Williston

Table 4. Minimum Required Road Setbacks in Williston’s Zoning Districts.

Zoning District	Setbacks from Streets and Roads			Side Yards	Rear Yards
	I-89	US 2 / VT 2A	Other Roads		
ARZD	150	50	50/25 (private)	15	15
RZD	150	50	50 (arterial)/25	10	15
VZD	150	50	25	15	15
MURZD	150	50	50 (arterial)	Chapter 23	Chapter 23
MUCZD	150	25	25	Chapter 23	Chapter 23
BPZD		50	50	Chapter 23	Chapter 23
TCZD	150	25	Build to	Chapter 23	Chapter 23
IZDW	150	35	35	Chapter 23	Chapter 23
IZDE			50	Chapter 23	Chapter 23
GZDS	150	75	50	Chapter 23	Chapter 23
GZDN		50	25	Chapter 23	Chapter 23

The remaining area was reduced by 17% for the IZDE zoning class and 14.5% for the other commercial zoning classes to account for additional yard and other landscaping setbacks. The resulting area equates to MIA at build-out for the commercial and industrial parcels.

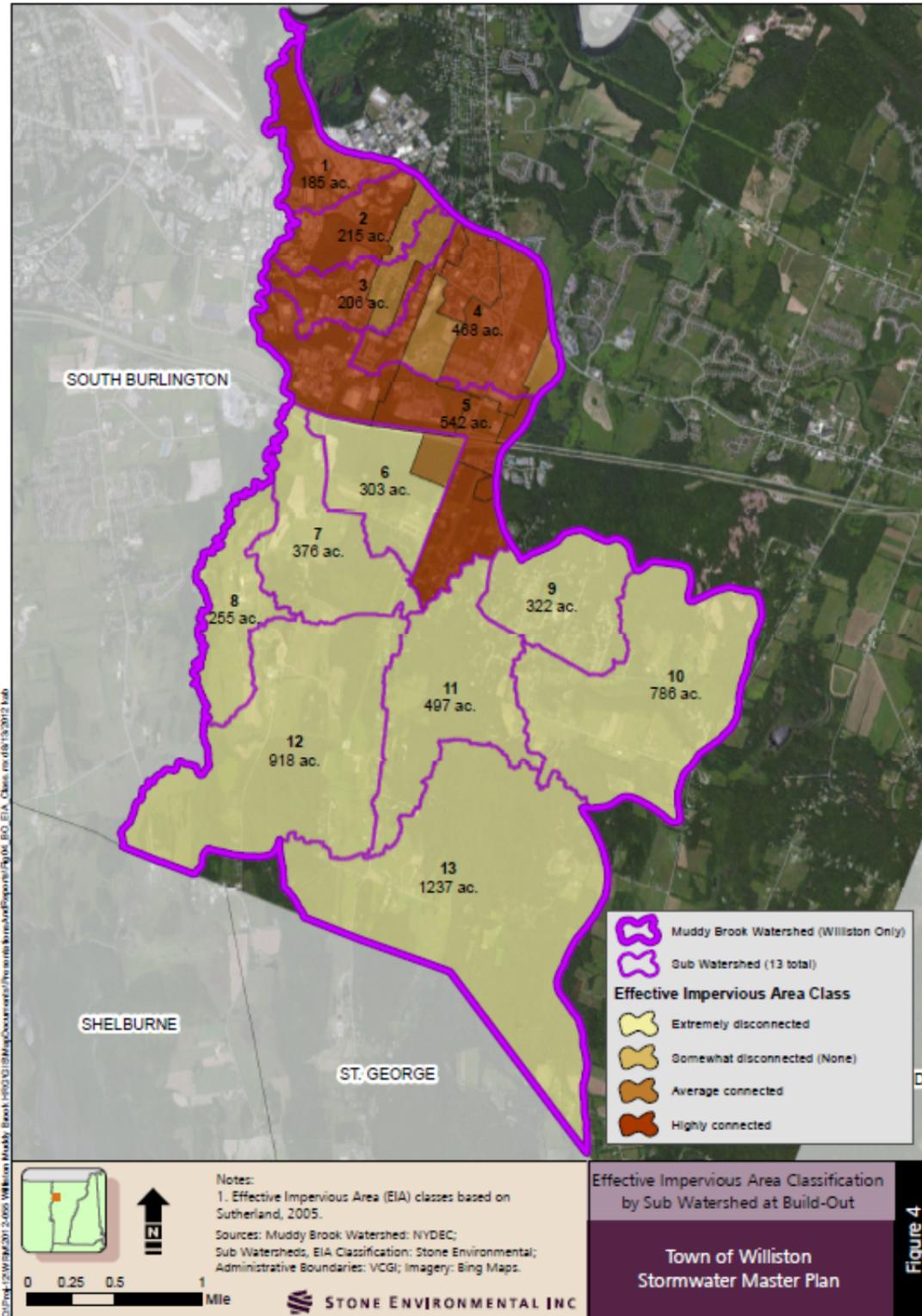
1.3.2 Estimating EIA at Build-Out

The residential MIA at build-out and commercial MIA at build-out were summarized by zoning class and sub-watershed. It was assumed that, at full build-out, sub-watersheds will have more stormwater infrastructure (e.g., storm sewers) than at the current condition in certain zoning classes. EIA classes were reassigned based on sub-watershed and zoning class district, for estimating EIA at build-out. See Table 5 for the Current Condition EIA Class and Build-Out EIA Class by sub-watershed and zoning class. Figure 4 illustrates the EIA classes by sub-watershed and zoning class.

Table 5. Effective impervious area (EIA) classes at current condition and build-out.

Watershed ID	District	DistrictAb	Current Condition EIA Class	Build-Out EIA Class
1	Industrial West Zoning District	IZDW	Highly connected	Highly connected
2	Industrial West Zoning District	IZDW	Average connected	Highly connected
2	Residential Zoning District	RZD	Average connected	Average connected
3	Industrial West Zoning District	IZDW	Average connected	Highly connected
3	Residential Zoning District	RZD	Average connected	Average connected
4	Business Park Zoning District	BPZD	Somewhat disconnected	Highly connected
4	Industrial West Zoning District	IZDW	Somewhat disconnected	Highly connected
4	Mixed Use - Commercial Zoning District	MUCZD	Somewhat disconnected	Average connected
4	Mixed Use - Residential Zoning District	MURZD	Somewhat disconnected	Average connected
4	Residential Zoning District	RZD	Somewhat disconnected	Average connected
4	Taft Corners Downtown Zoning District	TCZD	Somewhat disconnected	Highly connected
5	Agriculture/Rural Residential Zoning District	ARZD	Highly connected	Highly connected
5	Gateway South Zoning District	GZDS	Highly connected	Highly connected
5	Industrial West Zoning District	IZDW	Highly connected	Highly connected
5	Mixed Use - Commercial Zoning District	MUCZD	Highly connected	Highly connected
5	Mixed Use - Residential Zoning District	MURZD	Highly connected	Highly connected
5	Taft Corners Downtown Zoning District	TCZD	Highly connected	Highly connected
6	Agriculture/Rural Residential Zoning District	ARZD	Extremely disconnected	Extremely disconnected
6	Gateway South Zoning District	GZDS	Extremely disconnected	Average connected
6	Industrial West Zoning District	IZDW	Extremely disconnected	Highly connected
6	Mixed Use - Commercial Zoning District	MUCZD	Extremely disconnected	Average connected
7	Agriculture/Rural Residential Zoning District	ARZD	Extremely disconnected	Extremely disconnected
7	Industrial West Zoning District	IZDW	Extremely disconnected	Highly connected
8	Agriculture/Rural Residential Zoning District	ARZD	Extremely disconnected	Extremely disconnected
9	Agriculture/Rural Residential Zoning District	ARZD	Extremely disconnected	Extremely disconnected
10	Agriculture/Rural Residential Zoning District	ARZD	Extremely disconnected	Extremely disconnected
11	Agriculture/Rural Residential Zoning District	ARZD	Extremely disconnected	Extremely disconnected
12	Agriculture/Rural Residential Zoning District	ARZD	Extremely disconnected	Extremely disconnected
13	Agriculture/Rural Residential Zoning District	ARZD	Extremely disconnected	Extremely disconnected

Based on the EIA class and the estimated Residential and Commercial MIA at build-out, EIA at build-out was calculated and then summarized by sub-watershed. See Table 6 and Figure 5 for the change in EIA by sub-watershed.



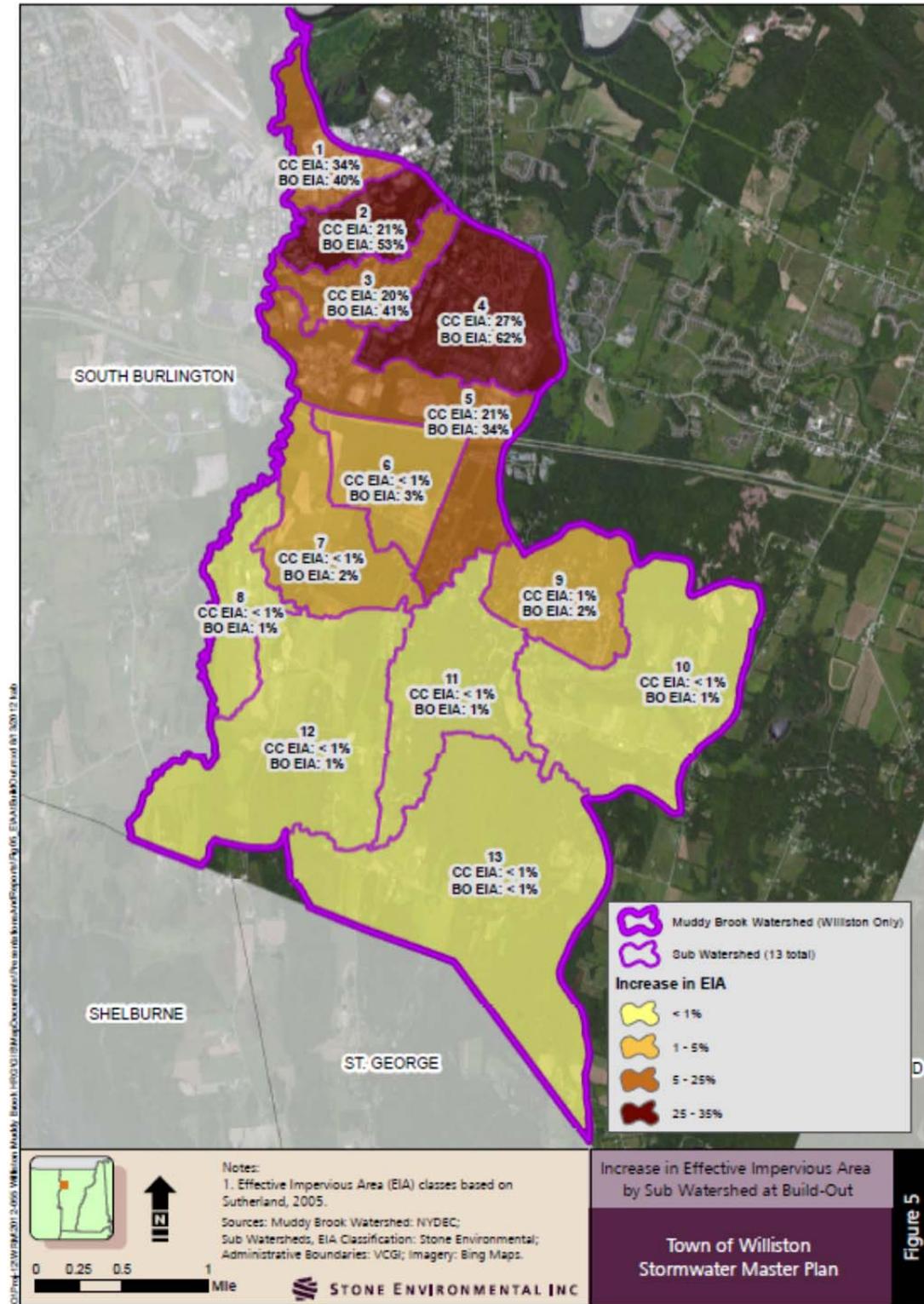


Table 6. Estimated EIA at current condition and build-out by sub-watershed.

Watershed ID	Total Acres	Current Condition EIA (%)	Build-Out EIA (%)	Increase in EIA (%)
1	184.9	33.5	39.9	6.4
2	215.2	20.9	53.3	32.4
3	206.3	19.5	41.4	21.9
4	467.8	27.2	62.2	35.0
5	542	21.2	34.3	13.1
6	302.6	0.4	2.5	2.2
7	375.9	0.2	1.7	1.5
8	254.7	0.1	1.1	1.0
9	322.4	1	2.3	1.2
10	786.5	0.1	1.1	0.9
11	497.5	0.1	0.8	0.7
12	917.8	0.1	0.9	0.8
13	1236.6	0	0.7	0.7

1.3.3 Estimating Stormwater Design Flow at Build-out

The calculated peak flow rates under current and build-out conditions are presented in Table 7. These values provide “ballpark” numbers for flow rates in each catchment but are not easily used to compare each sub-watershed with the others.

Table 7. Estimated peak flow rates at current condition and build-out by sub-watershed for 2, 10, 25, and 100-yr storm-events.

Sub-Watershed Catchment	2-yr Storm		10-yr Storm		25-yr Storm		100-yr Storm	
	Current acre-ft/hr	Built-Out acre-ft/hr	Current acre-ft/hr	Built-Out acre-ft/hr	Current acre-ft/hr	Built-Out acre-ft/hr	Current acre-ft/hr	Built-Out acre-ft/hr
1	6.4	7.2	7.1	8.0	7.7	8.7	8.7	9.8
2	5.2	9.4	5.7	10.4	6.2	11.3	7.0	12.8
3	5.8	8.6	6.4	9.4	6.9	10.3	7.8	11.6
4	15.1	24.6	16.6	27.0	18.2	29.5	20.5	33.3
5	15.9	20.0	17.5	22.0	19.1	24.1	21.5	27.2
6	5.3	6.0	5.9	6.6	6.4	7.2	7.3	8.1
7	7.4	8.0	8.2	8.9	8.9	9.7	10.1	10.9
8	6.1	6.4	6.7	7.1	7.4	7.7	8.3	8.7
9	5.7	6.4	6.2	7.1	6.8	7.7	7.7	8.7
10	14.9	15.8	16.4	17.4	17.9	19.0	20.2	21.4
11	8.7	9.2	9.5	10.1	10.4	11.0	11.7	12.4
12	19.2	20.1	21.1	22.1	23.0	24.2	26.0	27.3
13	21.9	22.7	24.1	25.0	26.4	27.3	29.8	30.8

Since the same rainfall intensity is applied across each sub-watershed catchment, comparisons can be simplified by removing the i-value from the calculation. This leaves the runoff coefficient (C) and Area (A) product which can be more easily displayed and compared as shown. The root of the expected change in hydrologic conditions is a shift from pervious to more impervious land cover. Figure 6 provides a comparison of current and build-out condition EIA and total sub-watershed catchment areas. Also shown in the secondary axis is the percent change between current and build-out EIA. The blue columns show total

catchment area. Sub-watersheds 1 through 5 show a significant fraction of land as EIA with varying degrees of change (provided in on the secondary axis). Sub-watersheds 6 through 13 show a very small, nearly undetectable, fraction as EIA in both current and build-out conditions. It should be noted that the relative change for these EIA values were very large - large enough to distort the scale of the figure. The change in current to build-out EIA is for sub-watersheds 6 through 13, however, is expected to result in very little change in hydrologic conditions. For these reasons, the values for “relative change” were omitted for sub-watersheds 6-13.

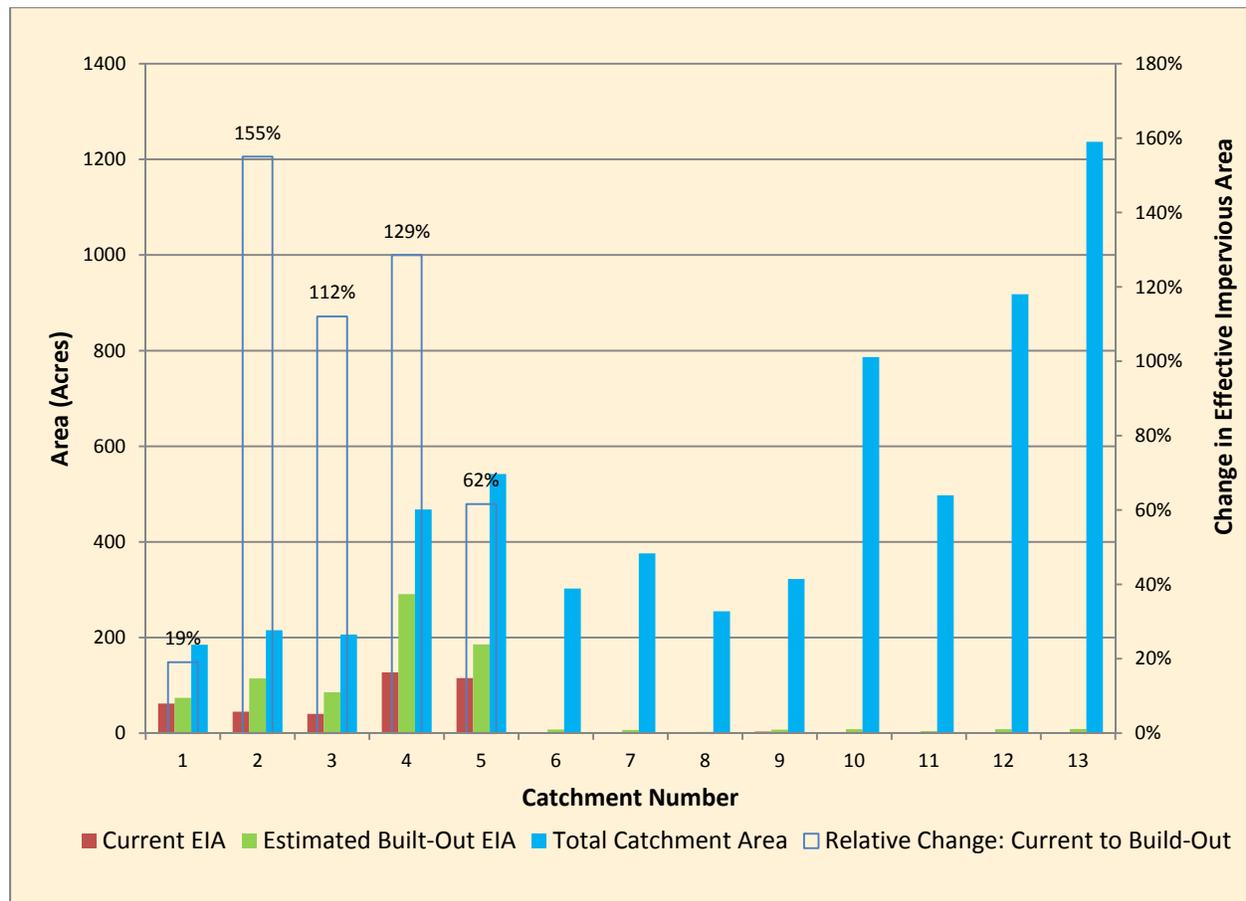


Figure 6. Comparison of current and build-out EIA, Total Catchment Area, and percent change.

The greatest potential for change can be expected in the sub-watershed catchments 2, 3, 4, and 5. This is the commercial district north of Interstate 89 (Tafts Corners). When compared to the catchments in the southern portion of the watershed, these areas are much smaller in area but are more highly-developed. Figure 7 factors in the land-use/land-cover type and assesses the associated runoff contributions by including the C-values for each sub-watershed catchment. Because the C-value relates the amount of runoff to the amount of precipitation received (i.e., it is a larger value for areas with low infiltration and high runoff such as pavement, and lower for permeable, well vegetated areas such as forests), the C x A product provides a way to compare the total relative runoff contributions from each sub-watershed. The figure shows contributions from current and build-out EIA and pervious areas.

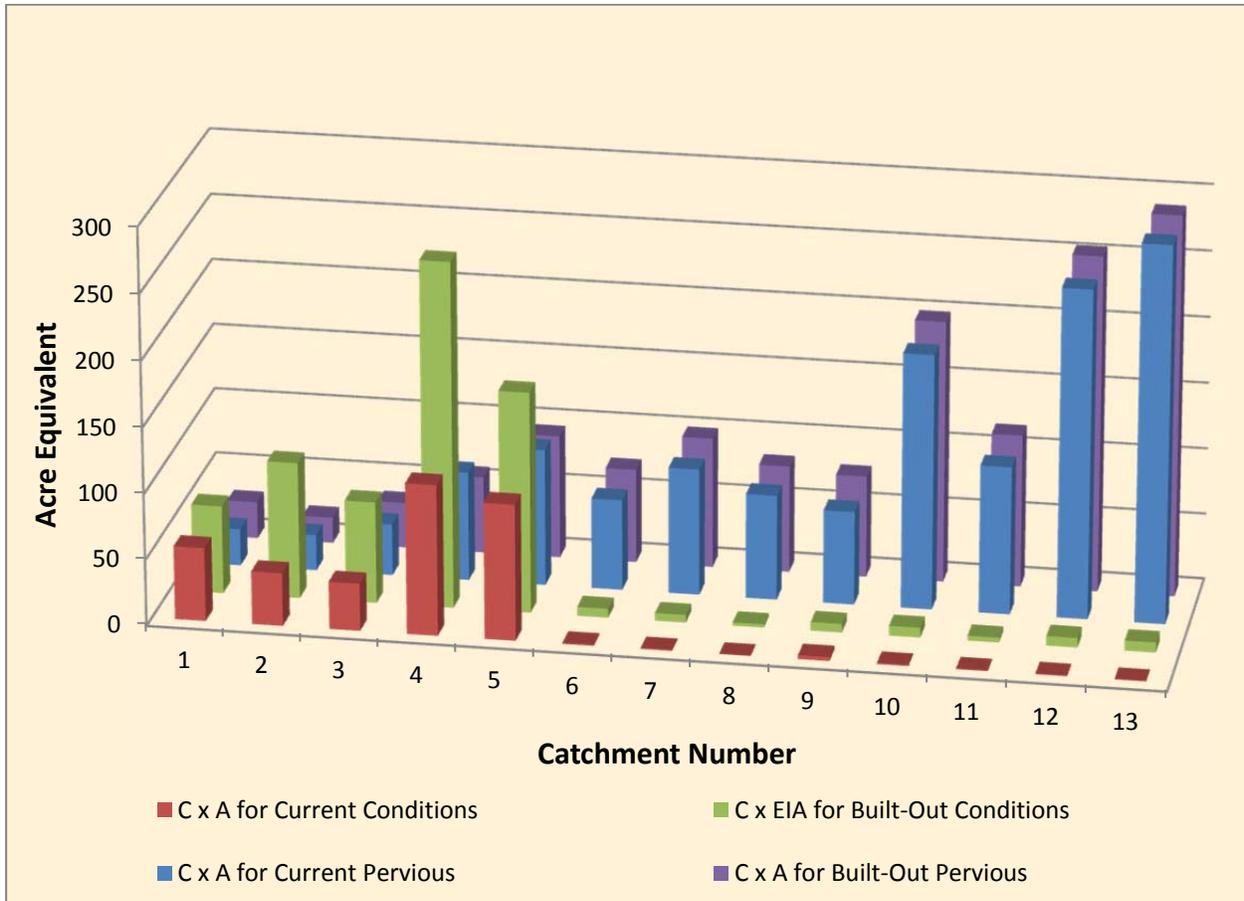


Figure 7. Comparison of Runoff Coefficient x Areas

Considering both Figure 6 and 7, one can see that while sub-watershed catchments 1 through 5 make up only a small part of the total watershed area, they contribute a disproportionately large amount of runoff under both current and build-out conditions. This is due to the large amount of impervious area associated with these highly-developed parts of Williston.

Another way to assess and compare hydrologic conditions across sub-watershed catchments and between current and build-out conditions is to look at runoff flow rates on a per-acre basis as show in Figure 8. A higher flow per acre value means flashier flows, which are more likely to lead to erosion in ditches, tributaries and mainstem streams.

Figure 8 shows higher intensity flow rates per acre of land for sub-watersheds 1 through 5. These are the most highly developed areas within the watershed, as well as where future development will be concentrated. This means that that the ditches and small streams in these areas are likely to already be significantly more stressed during a typical storm event by higher flows. Furthermore, when fully built out, flow rates per acre are expected to significantly increase.

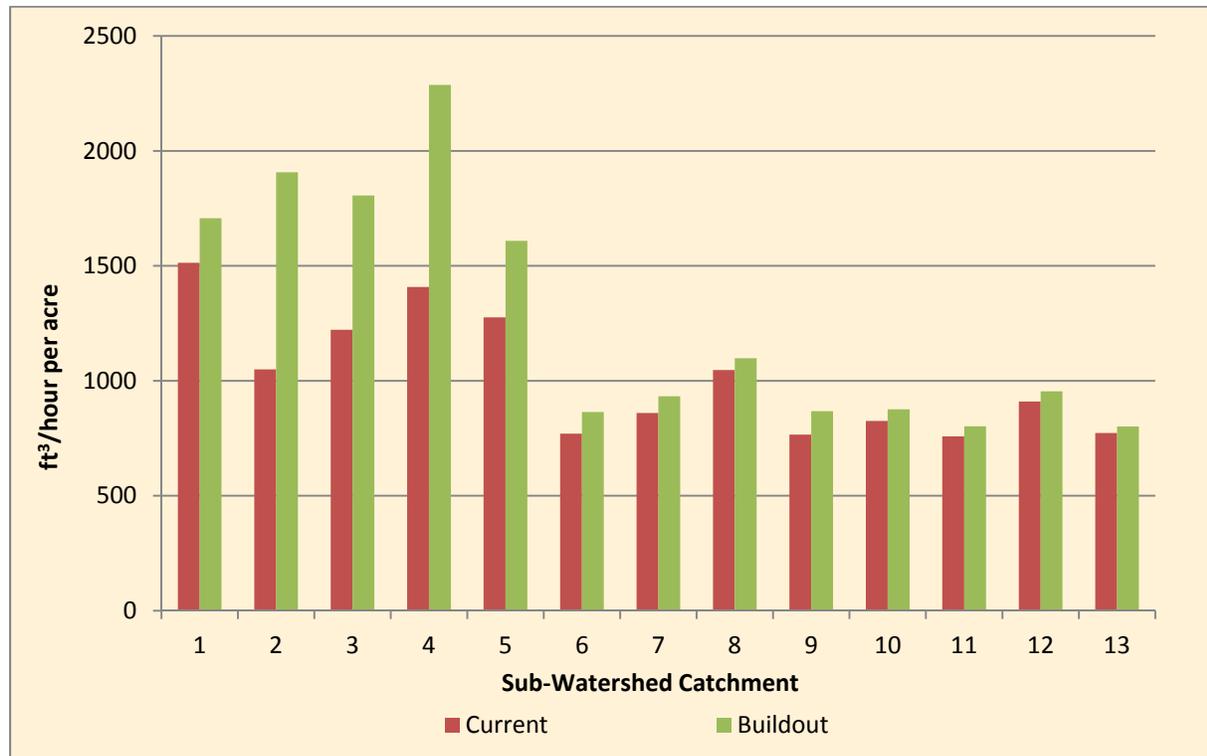


Figure 8. 2-yr/ 140 minute flow rates are shown as flow per acre for current and build-out conditions.

2.0 CONCLUSIONS

For future planning and stormwater management purposes the Town may wish to consider zoning conditions, low-impact design requirements, and other measures to help manage stormwater runoff – particularly from parcels that do not reach the regulatory threshold for state stormwater jurisdiction. Stormwater treatment retrofits could provide significant water quality benefits if implemented in sub-watershed 1 followed by 4, 5, 3 and 2 respectively. Low impact design could help to minimize stormwater impacts from new, sub-jurisdictional development in areas expecting to see the most growth. Based on our build-out analysis these are subwatersheds 2 and 4 followed by 3, 5, and 1 respectively.

The less developed sub-watersheds - 6 through 13 - should not be ignored, however. These sub-watersheds account for the majority of total area and in absolute terms contribute the greatest amount of runoff to Muddy Brook. A large portion of these areas are currently forested. From a water quality standpoint it is important to protect and sustain these valuable areas. Much of the remaining land is used for agriculture, mostly classified as hay fields and pasture-land, but also some annual row crops. In pasture areas efforts to exclude animals from streams and to create and maintain riparian buffers can significantly reduce erosion and nutrient runoff. Likewise conservative tillage practices can prevent erosion from annual cropland while helping to maintain soil quality.